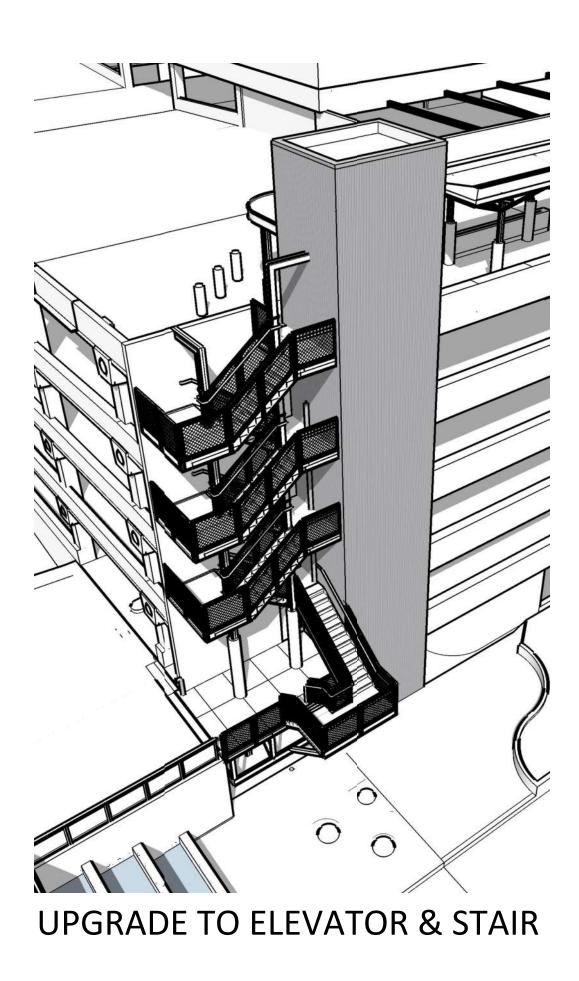


EXISTING ELEVATOR & STAIR



SDCI DEVELOPMENT SITE

PARCEL NUMBER: 6598350000

SDCI LEGAL DESCRIPTION:
PC-1 SOUTH CONDOMINIUM, VOL 100, PAGES 44-50 AKA PAR A, 8800103,
LTS 1-8, BLK H, A.A. DENNY'S 4TH ADD, EXC POR LT 2 CONDEMNED, EXC POR

FOR ARMORY WY IN SUPERIOR COURT # (FILE)

SITE ADDRESS

1531 WESTERN AVENUE SEATTLE, WA 98101

BUILDING OWNER

PIKE PLACE MARKET PDA 85 PIKE STREET, ROOM 500 SEATTLE, WA 98101

LEGAL DESCRIPTION

FULL LEGAL DESCRIPTION OF CONDOMINIUM

LOTS 1, 2, 3, 4, 5, 6, 7 AND 8 IN BLOCK H OF ADDITION TO THE TOWN OF SEATTLE AS LAID OUT BY A.A. DENNY (COMMONLY KNOWN AS A.A. DENNY'S 4TH ADDITION TO THE CITY OF SEATTLE), AS PER PLAT RECORDED IN VOLUME 1 OF PLATS, PAGE 69, RECORDS OF KING COUNTY;

EXCEPT THAT PORTION OF SAID LOT 2 CONDEMNED FOR WIDENING AND EXTENSION OF WESTERN AVENUE PURSUANT TO ORDINANCE NO. 18109 OF THE CITY OF SEATTLE;

AND EXCEPT THAT PORTION TAKEN FOR ARMORY WAY IN KING COUNTY SUPERIOR COURT CAUSE NO. 292884, DESCRIBED AS THAT PORTION OF LOTS, 1, 4, 5 AND 8 IN BLOCK H LYING SOUTHWESTERLY OF A LINE 31.25 FEET SOUTHWESTERLY FROM AND PARALLEL WITH THE SOUTHWESTERLY MARGIN OF THE ALLEY AS PLATTED IN SAID BLOCK H;

TOGETHER WITH THAT PORTION OF THE ALLEY IN SAID BLOCK H AS VACATED BY ORDINANCE NO. 107097 LYING NORTHWESTERLY OF THE SOUTHEASTERLY LINE EXTENDED OF LOTS 7 AND 8 IN SAID BLOCK H; AND THAT PORTION OF PINE STREET AS VACATED BY ORDINANCE NO. 23613 AND ORDINANCE NO. 107097 LYING BETWEEN THE NORTHWESTERLY LINE OF SAID BLOCK H AND A LINE PARALLEL TO AND 30 FEET NORTHWESTERLY OF THE NORTHWESTERLY LINE OF SAID BLOCK H AND THAT PORTION OF THE NORTHEASTERLY 15 FEET OF ELLIOTT AVENUE (ALASKAN FREEWAY, FORMERLY ARMORY WAY) AS VACATED BY ORDINANCE NO. 114330 LYING BETWEEN A LINE 140 FEET NORTHWESTERLY OF THE NORTHWESTERLY LINE OF PIKE STREET (BEING THE PRODUCTION SOUTHWESTERLY OF THE SOUTHEASTERLY LINE OF LOT 8 IN BLOCK H OF SAID A.A. DENNY'S 4TH ADDITION) AND A LINE 30 FEET NORTHWESTERLY OF AND PARALLEL WITH THE NORTHWESTERLY LINE OF SAID BLOCK H;

SITUATE IN THE CITY OF SEATTLE, COUNTY OF KING, STATE OF WASHINGTON.

PROJECT DESCRIPTION

UPGRADE TO EXISTING ELEVATOR AND EXISTING EXTERIOR STAIRS AT SW CORNER OF EXISTING MARKET PARKING GARAGE. NO CHANGES TO OCCUPANCY OR USE.

CODE: 2018 SEATTLE BUILDING CODE
2018 SEATTLE EXISTING BUILDING CODE
2018 SEATTLE ENERGY CODE
ELEVATOR CODE OF THE CITY OF SEATTLE

CONSTRUCTION TYPE: 1-B EXISTING PARKING GARAGE OCCUPANCY: S-2, OPEN PARKING GARAGE, FULLY-SPRINKLERED

RELATED PERMIT: 6747735-CN

GENERAL NOTES

- 1. EVERY CONDITION OR ITEM TO BE REMOVED IS NOT SHOWN. CONTRACTOR SHALL VERIFY ALL CONDITIONS.
- 2. RUBBISH AND DEBRIS TO BE REMOVED PROMPTLY FROM THE SITE. LOCAL REGULATIONS REGARDING HAULING AND DISPOSAL OF RUBBISH AND DEBRIS APPLY. FOLLOW CITY OF SEATTLE REQUIRED WASTE DIVERSION PLAN AND PROVIDE ON REQUEST ALL REQUIRED DOCUMENTATION. COORDINATE WITH ARCHITECT.
- COMPLY WITH APPLICABLE CODES AND REGULATIONS FOR DEMOLITION OPERATIONS AND SAFETY OF ADJACENT STRUCTURES AND THE PUBLIC.
- 4. COMPLY WITH APPLICABLE REQUIREMENT OF NFPA 241.
- TAKE PRECAUTIONS TO PREVENT CATASTROPHIC OR UNCONTROLLED COLLAPSE OF STRUCTURES TO BE REMOVED.
- PROVIDE, ERECT, AND MAINTAIN TEMPORARY BARRIERS AND SECURITY DEVICES.
 CONDUCT OPERATIONS TO MINIMIZE EFFECTS ON AND INTERFERENCE WITH ADJACENT STRUCTURE.
- 8. PROTECT EXISTING STRUCTURES AND OTHER ELEMENTS THAT ARE NOT TO BE REMOVED.
- 9. HAZARDOUS MATERIALS: COMPLY WITH 29 CFR 1926 AND STATE AND CITY OF SEATTLE REGULATIONS.
- PERFORM DEMOLITION IN A MANNER THAT MAXIMIZES SALVAGE AND RECYCLING OF MATERIALS.
- 11. ALL ACCESSIBILITY DESIGN AND CONSTRUCTION SHALL COMPLY WITH ICC/ANSI 117.1-2009.

ELEVATOR NOTES

- SBC SECTION 3022 AND ASME SECTIONS 2.7 AND 2.8. PIPES, DUCTS, CONDUITS, AND EQUIPMENT NOT USED FOR THE OPERATION OF THE ELEVATORS ARE PROHIBITED IN MACHINE ROOM AND HOISTWAYS.
- SBC 3020. MAINTAIN ALL REQUIRED WORKING CLEARANCES IN MACHINE ROOM.
 ASME RULE 2.2.2. WATERPROOF AS NECESSARY TO PREVENT ENTRY OF GROUND WATER.
 SUMP PUMPS MAY BE INSTALLED FOR FLOOD CONTROL BUT NOT APPROVED TO MAINTAIN
- 4. SBC 3023, ASME RULE 2.2.4. PROVIDE PIT LADDER.
- 5. ASME RULE 2.7.5.2. PROVIDE MACHINE ROOM VENTILATION.
 6. SBC 3016.5. PROVIDE HOISTWAY VENTILATION. PROVIDE MOTORIZED DAMPERS AS
- REQUIRED BY SEATTLE ENERGY CODE 1412.4.1 FOR ALL HOISTWAY VENTS.

 7. SBC 3016.3. COMPLY WITH SEISMIC REQUIREMENTS.
- 8. ASME RULE 2.7.4. PROVIDE 7'-0' CLEAR HEADROOM IN MACHINE ROOM.
- SBC 3016.4. AND CHAPTER 11; ACCOMMODATE PEOPLE WITH DISABILITIES.
 ASME SECTION 2.4 AND 3.4. PROVIDE PROPER TOP CAR RUNBYS, CLEARANCES AND REFUGE SPACE.
- 11. ASME RULE 2.1.1.2 AND 2.11.14. GROUT ALL MASONRY JAMBS AND HEADERS TO RETAIN FIRE RATING OF HOISTWAY. IN OTHER THAN MASONRY, PROVIDE LABELED ENTRANCE ASSEMBLIES INSTALLED AS TESTED.
- 12. SBC 3020. GROUT BEHIND ALL HOISTWAY PENETRATIONS FOR PIPES, FIXTURES, ETC.

 13. SBC 3016.5 ELEVATOR HOISTWAYS SHALL NOT BE VENTED OR PRESSURIZED THROUGH
- ELEVATOR MACHINE ROOMS.
- 14. SBC 3016.5.4 VENTILATION AND PRESSURIZATION EQUIPMENT, DUCTS, ETC. CANNOT BE LOCATED IN ELEVATOR MACHINE ROOMS, HOISTWAYS, OR SPACES.
- 15. ASME RULES 2.1.1.2 AND 2.14.1.8 GLASS USED IN OR ON ELEVATOR HOISTWAYS AND CARS MUST BE LAMINATED AND MEET THE REQUIREMENTS OF ASME 297.1
- 16. SBC 106 PROVIDE CALCULATIONS AND DRAWINGS TO SDCI FOR APPROVAL OF THE STRESSES AS NOTED IN THE APPLICABLE RULES OF ASME SECTION 2.9.
 17. ASME SECTION 2.6. PROVIDE CALCULATIONS TO SDCI FOR APPROVAL OF THE ABILITY OF THE
- PIT FLOOR AND STRUCTURE TO WITHSTAND THE ELEVATOR BUFFER ENGAGEMENT REACTIONS.
- 18. ASME 2.27.1. PROVIDE MEANS OF TWO-WAY CONVERSATION BETWEEN EACH ELEVATOR AND A READILY ACCESSIBLE POINT (MAIN ELEVATOR LOBBY) OUTSIDE THE HOISTWAY.
- ASME 2.27.1.1.2 THIS STRUCTURE IS CONSIDERED AS UNATTENDED, AND AN ADDITIONAL EMERGENCY SIGNALING DEVICES SHALL BE PROVIDED (PHONE TO ANSWERING SERVICE).
- 20. ASME 2.27.1.1.5 PROVIDE AN EMERGENCY POWER SUPPLY FOR THE DEVICES REQUIRED BY 2.27.1 THE SUPPLY SHALL BE CAPABLE OF OPERATING THE AUDIBLE DEVICE FOR AT LEAST ONE HOUR AND THE MEANS OF A TWO-WAY CONVERSATION FOR AT LEAST FOUR HOURS.
- SBC 3016.9. INSTALL APPROVED KEY RETAINER BOX, KEYED TO THE SECURE CITY KEY.
 SBC 3016.10 KEYS REQUIRED FOR THE OPERATION OF ELEVATOR, FIRE EMERGENCY SERVICE, THE MACHINE ROOM AND THE MECHANICAL HOISTWAY ACCESS KEY SHALL BE TAGGED AND
- KEPT IN THE KEY BOX.
 23. COMPLY WITH APPLICABLE CODES.
- 23. COMPLY WITH APPLICABLE CODES.

 24. ALL APPLICABLE ASME CODES, SEATTLE BUILDING CODES, SEATTLE ELECTRICAL CODES AND
- ELEVATOR CODES ADOPTED BY REFERENCE APPLY.
- 25. ELEVATOR PRIMARY RECALL FLOOR TO BE DESIGNATED AS LEVEL 2, ALTERNATE RECALL

SHEET INDEX:

EGRESS PLAN

ARCHITECTURAL
A0.1 COVER SHEET, NOTES & SITE PLAN

A0.4 TEMPORARY SEQUENCING / EGRESS PLAN

A2.0 LEVEL 1 PLAN A2.1 LEVEL 2 PLAN

A2.2 LEVEL 3 PLAN

2.3 LEVEL 4 PLAN

A2.4 LEVEL 5 PLAN A2.5 LEVEL 6 PLAN

A2.6 DOOR & PARTITION SCHEDULES A3.0 ELEVATIONS

3.1 ELEVATIONS 7.0 SECTIONS

8.0 DETAIL

DETAILS DETAILS

3 DETAILS

A8.4 DETAILS A8.5 DETAILS

A8.5 DE

STRUCTURAL S1.0 G

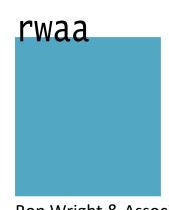
GENERAL NOTES GENERAL NOTES GENERAL NOTES

DETAILS

2.0 PLANS 4.0 DETAILS 4.1 DETAILS

VERTICAL TRANSPORATION

VT-1 VT-2



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PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



Date Issue / Revision

06/03/21 PERMIT SET

9/20/21 UPDATED REVIEW SET

12/10/21 CONTRACTOR PRICING SET

CONTENTS:

COVER SHEET NOTES SITE PLAN

SCALE: AS NOTED

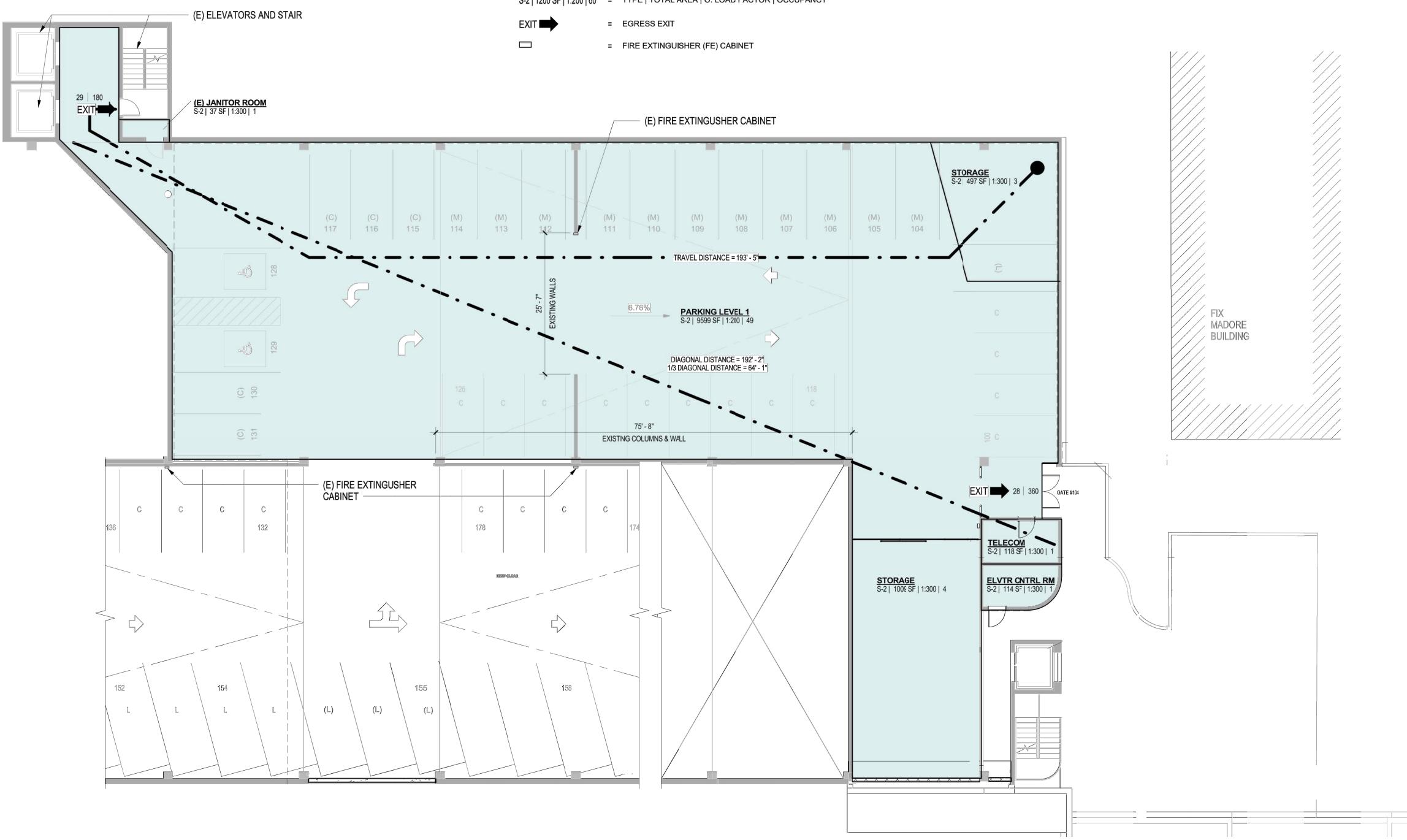
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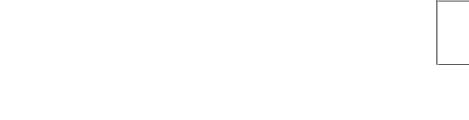
| OCCUPANCY - LEVEL ONE | | | | | | | | |
|---|----------|-----|-----|----|--|--|--|--|
| Name Area Occupancy Group Occupancy Load Factor | | | | | | | | |
| PARKING LEVEL 1 | 9625 SF | S-2 | 200 | 49 | | | | |
| STORAGE | 1009 SF | S-2 | 200 | 6 | | | | |
| TELECOM | 115 SF | S-2 | 200 | 1 | | | | |
| STORAGE | 495 SF | S-2 | 200 | 3 | | | | |
| Grand total | 11245 SF | | | 59 | | | | |

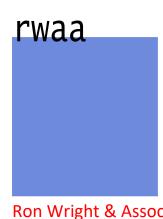
EGRESS PLAN LEGEND:

S-2 | 1200 SF | 1:200 | 60 = TYPE | TOTAL AREA | O. LOAD FACTOR | OCCUPANCY



LEVEL 1 EGRESS PLAN





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PC-1S **ELEVATOR** AND STAIR **REPLACEMENT**

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



EGRESS PLANS

FILE: . SHEET:

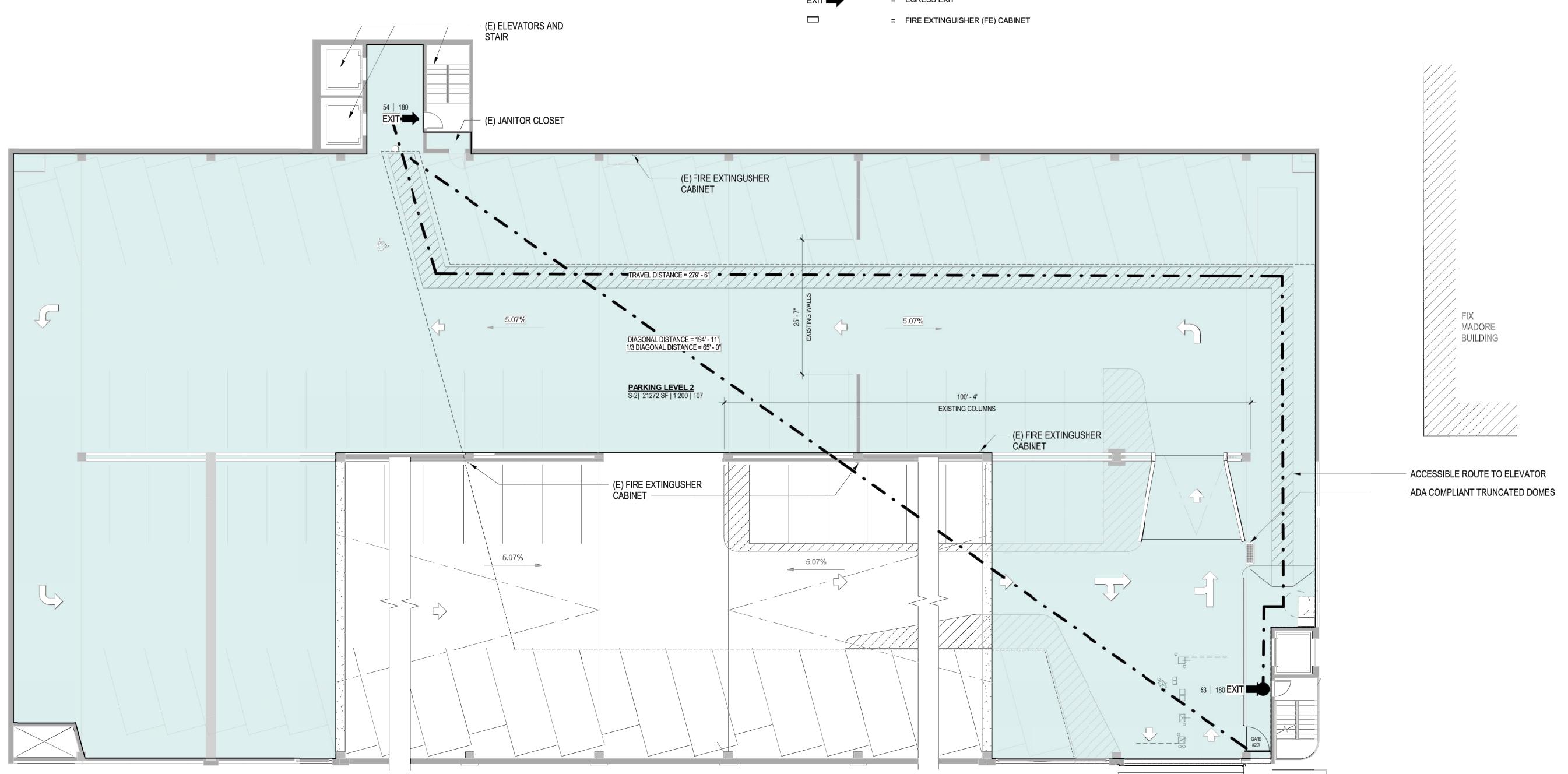
| OCCUPANCY - LEVEL TWO | | | | | | | | |
|-----------------------|----------|-----------------|-----------------------|-----------|--|--|--|--|
| Name | Area | Occupancy Group | Occupancy Load Factor | Occupants | | | | |
| PARKING LEVEL 2 | 21272 SF | S-2 | 200 | 107 | | | | |
| Grand total | 21272 SF | | | 107 | | | | |

EGRESS PLAN LEGEND:

= ACTUAL ASSIGNED DOOR LOAD | DOOR CAPACITY*

S-2 | 1200 SF | 1:200 | 60 = TYPE | TOTAL AREA | O. LOAD FACTOR | OCCUPANCY

= EGRESS EXIT



LEVEL 2 SHOWN, LEVELS 3, 4 & 5 ARE SIM WITHOUT ENTRY DRIVE.

2. LEVEL 6 IS OPEN TO WESTERN AVE.

1 LEVEL 2-5 EGRESS PLAN NTS

PC-1S **ELEVATOR** AND STAIR REPLACEMENT

rwaa

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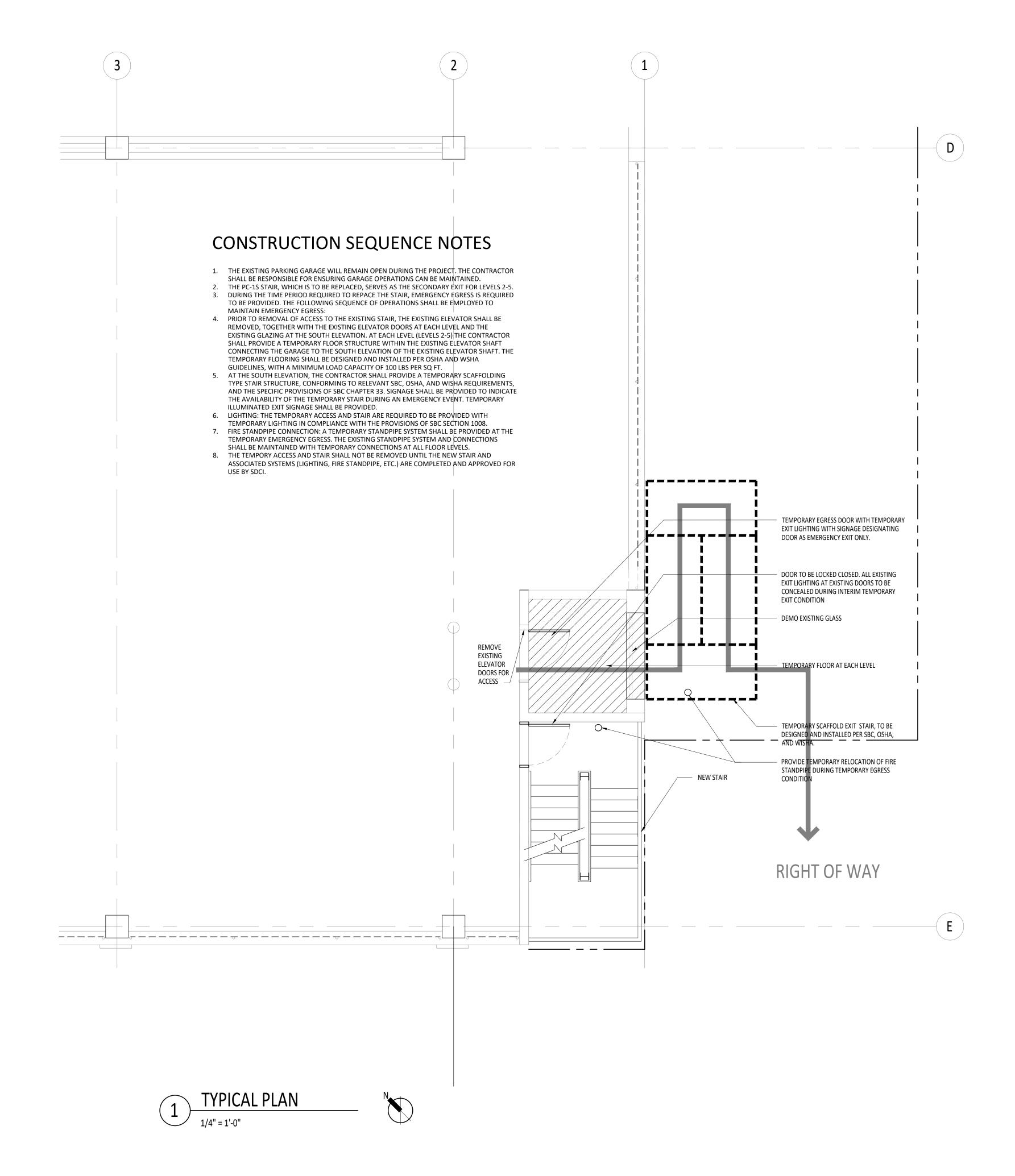


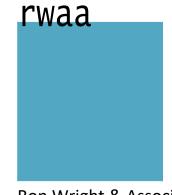
Date Issue / Revision 06/03/21 PERMIT SET

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EGRESS PLANS

SCALE: AS NOTED PROJECT NO.: 20020 FILE: SHEET:





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PC-1S ELEVATOR AND STAIR REPLACEMENT

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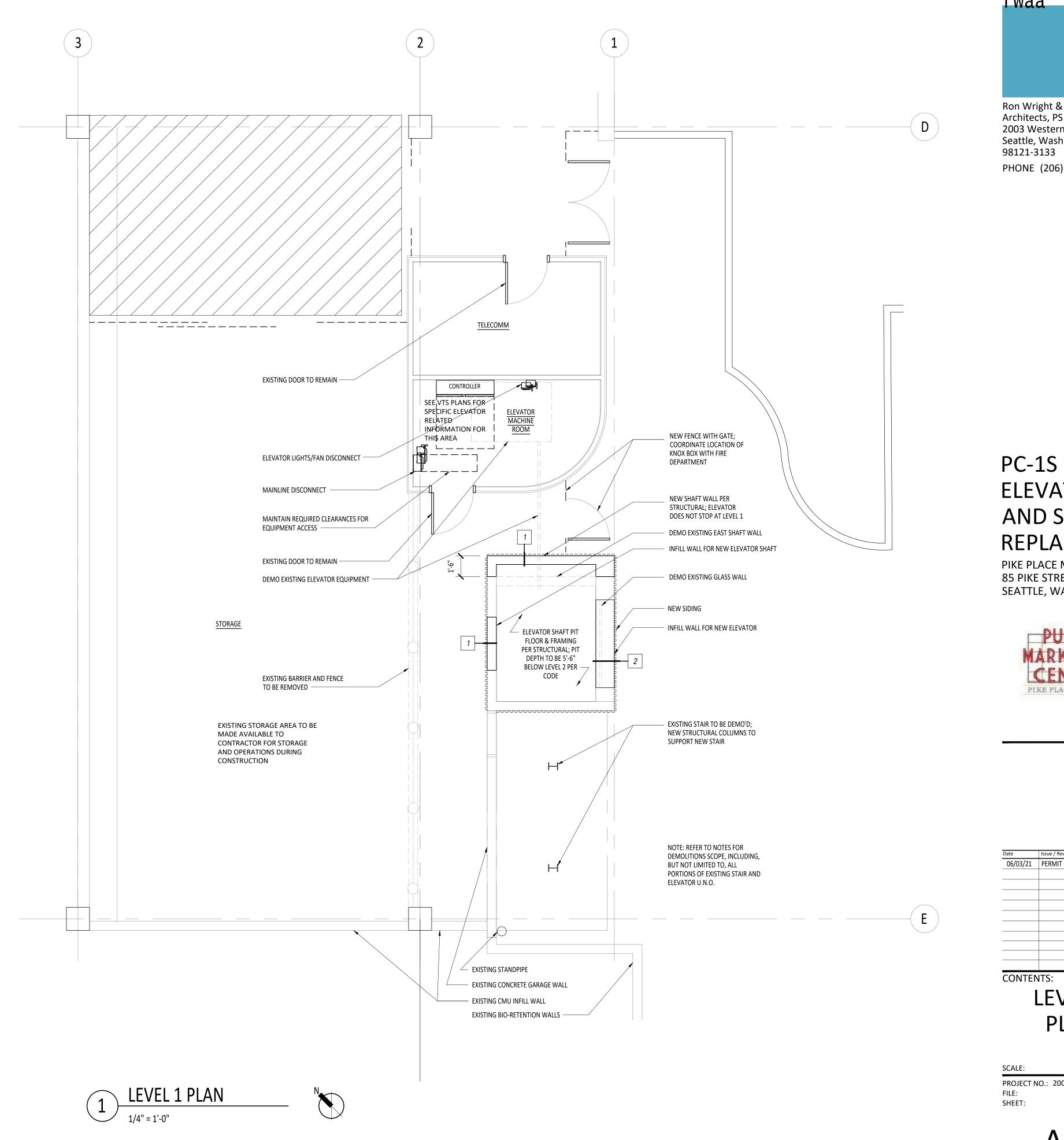


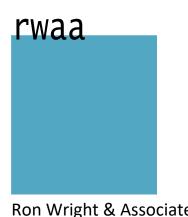
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SEQUENCING TEMPORARY

EGRESS

PROJECT NO.: 20020 FILE:





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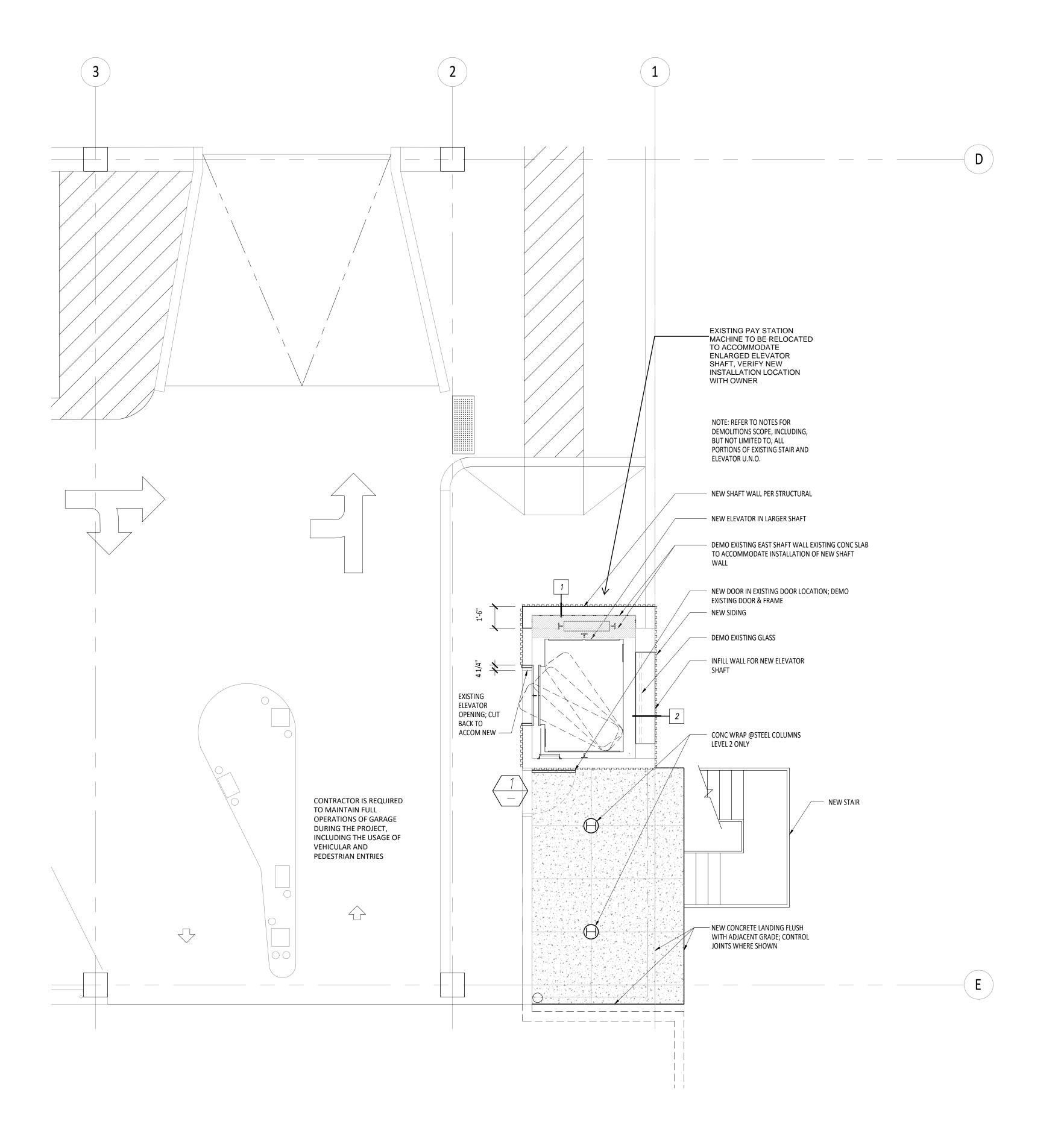
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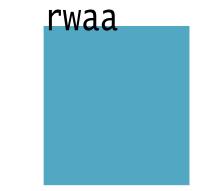
PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



LEVEL 1 PLAN

PROJECT NO.: 20020 FILE:





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PC-1S ELEVATOR AND STAIR REPLACEMENT

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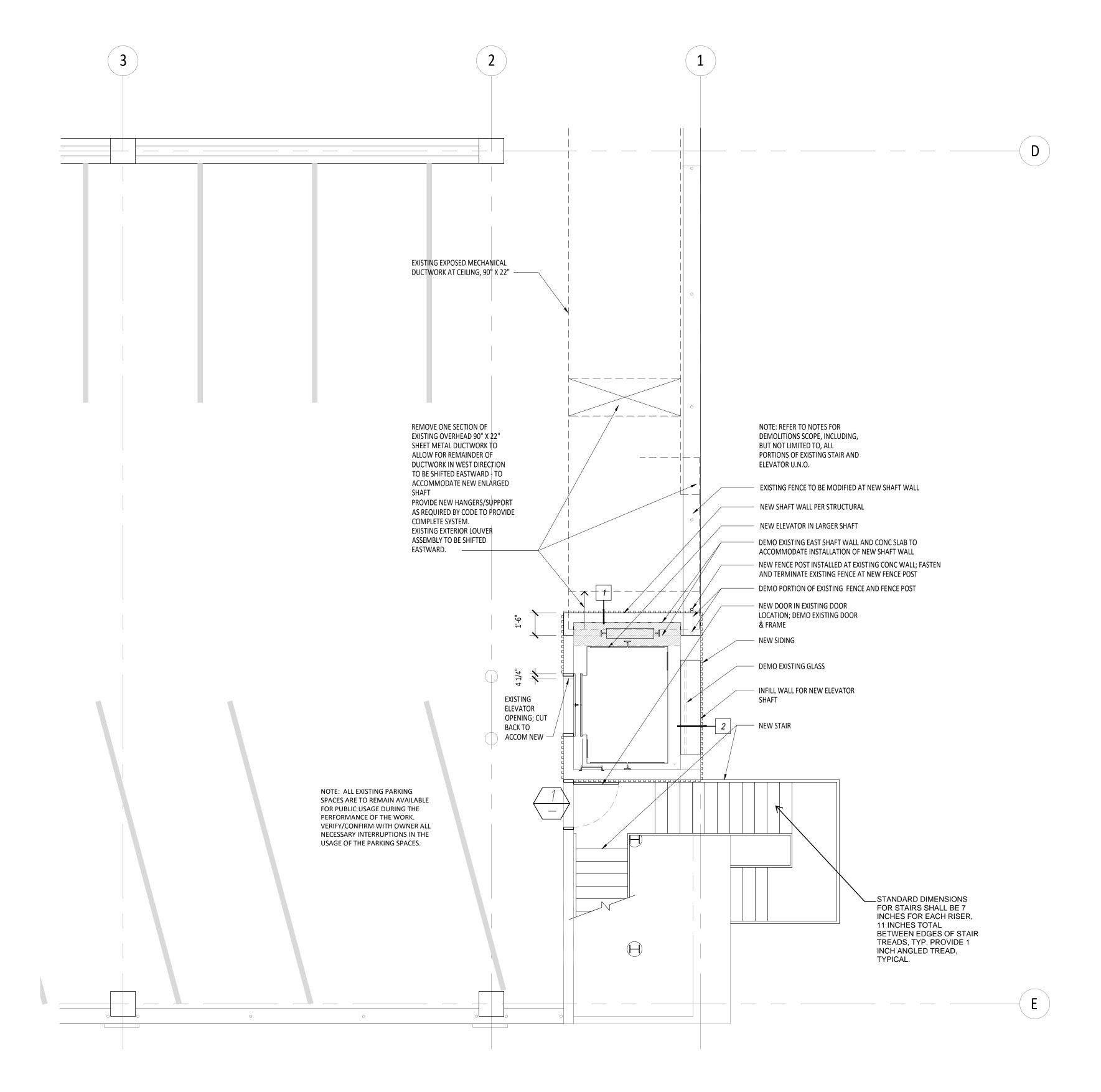
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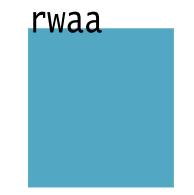
LEVEL 2 PLAN

SCALE:
PROJECT NO.: 20020
FILE:
SHEET:

A2.1





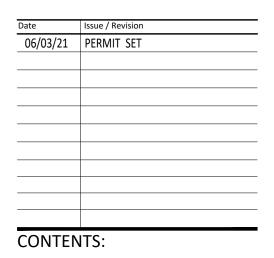


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PC-1S **ELEVATOR** AND STAIR REPLACEMENT

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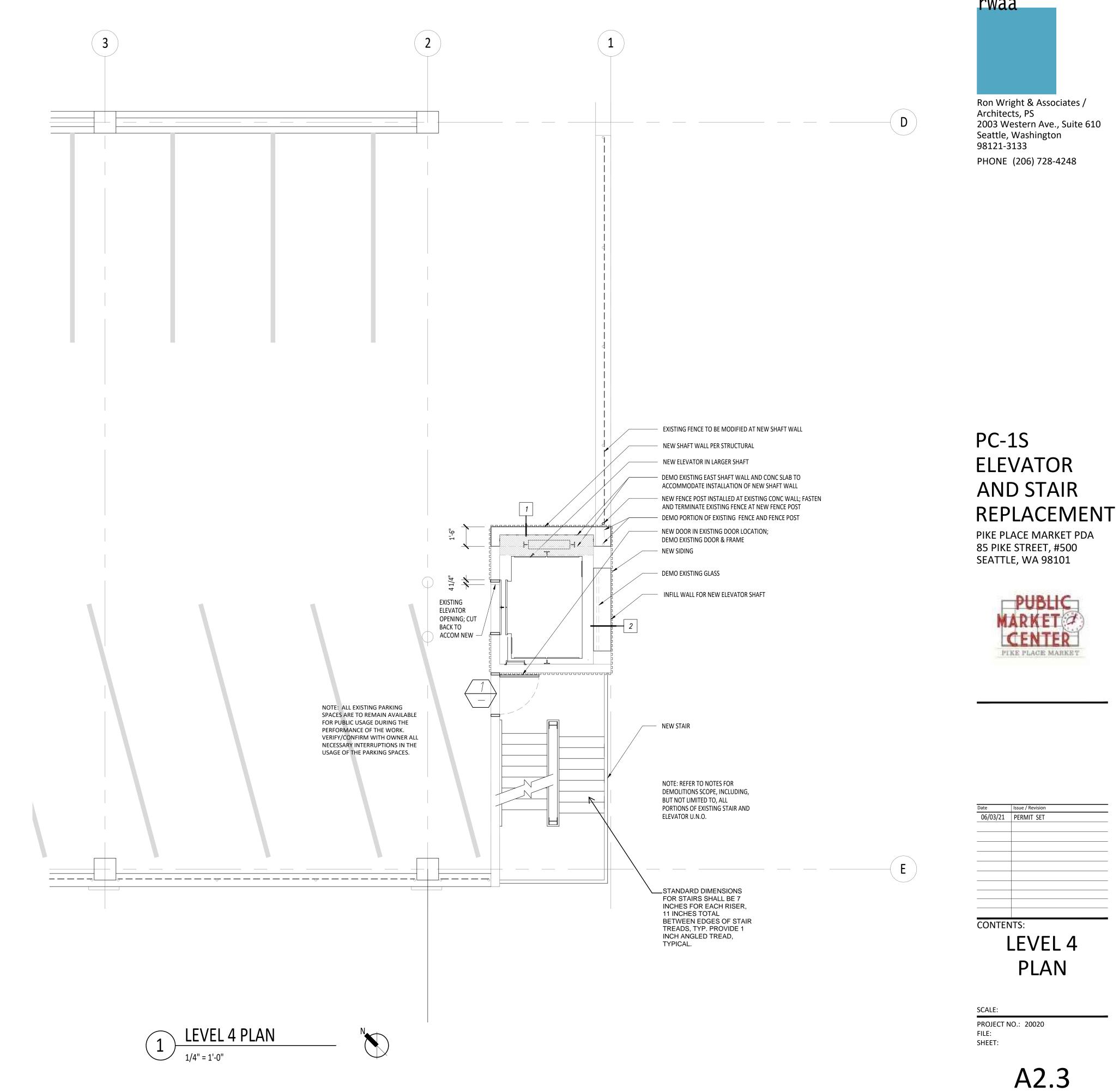


LEVEL 3 PLAN

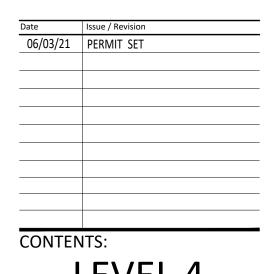
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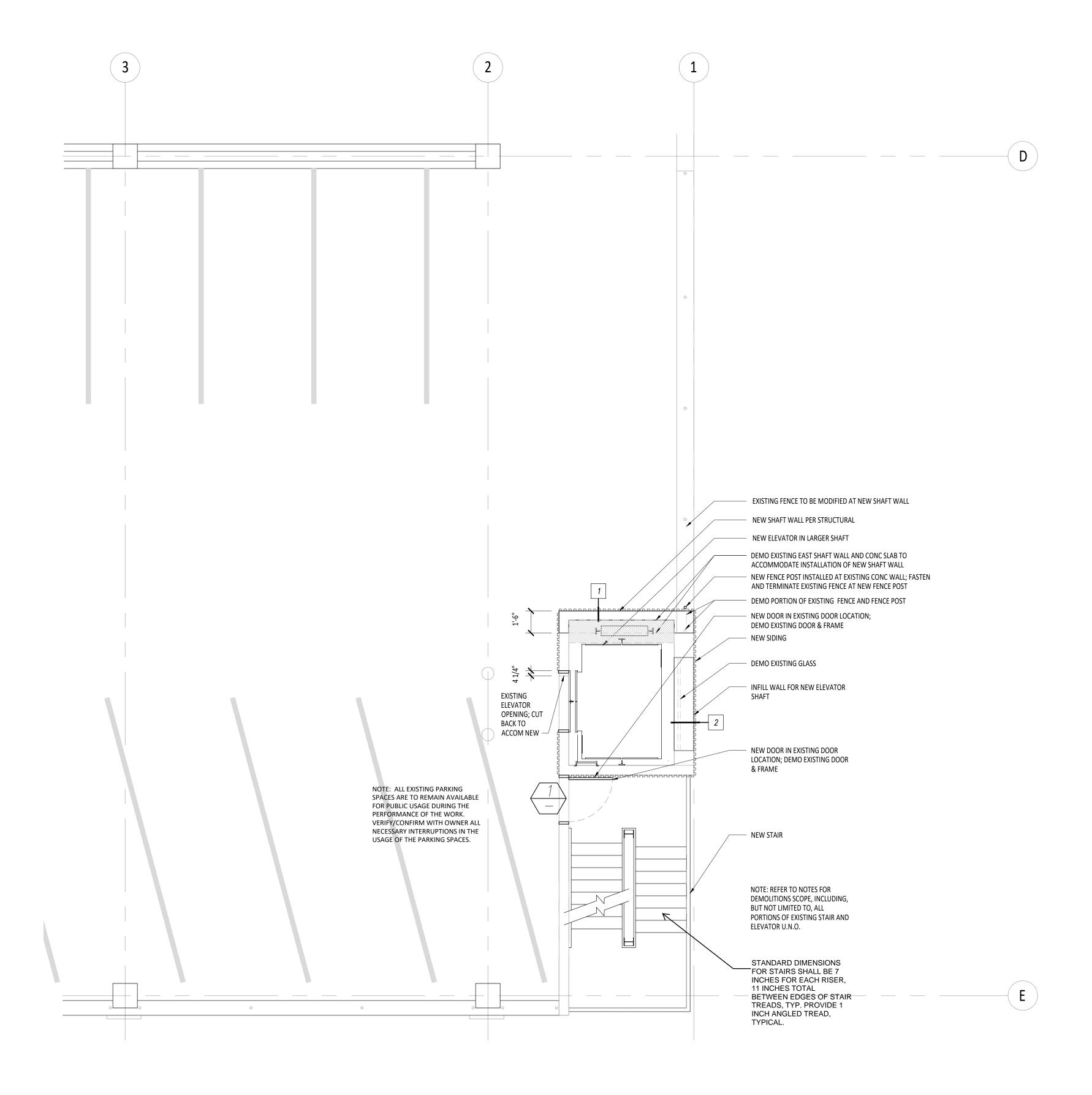
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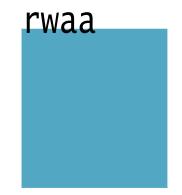












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PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



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CONTENTS:

LEVEL 5
PLAN

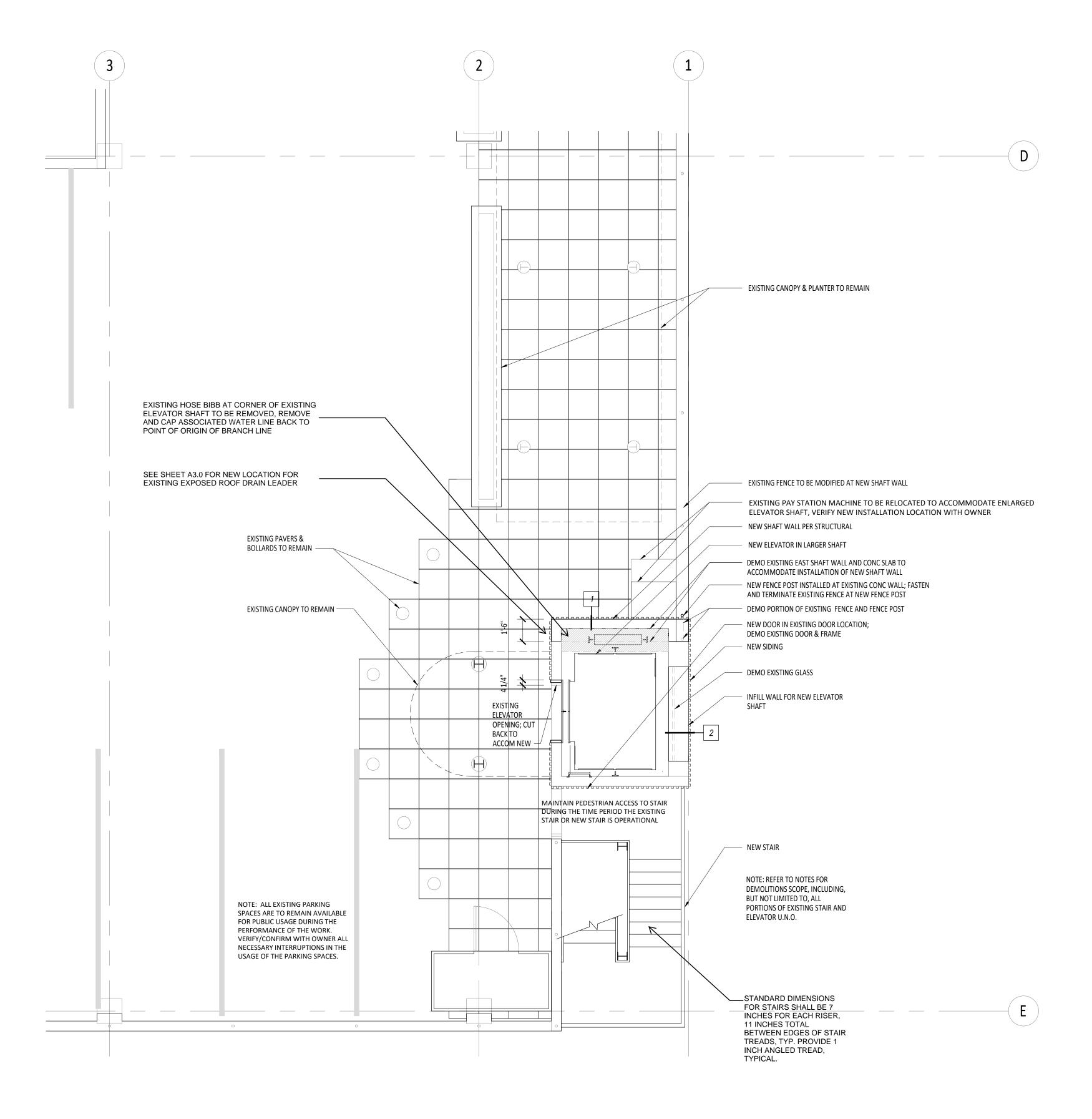
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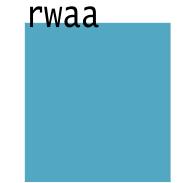
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A2.4

1 LEVEL 5 PLAN

1/4" = 1'-0"





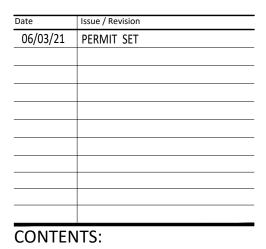
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PC-1S **ELEVATOR** AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101





LEVEL 6 PLAN

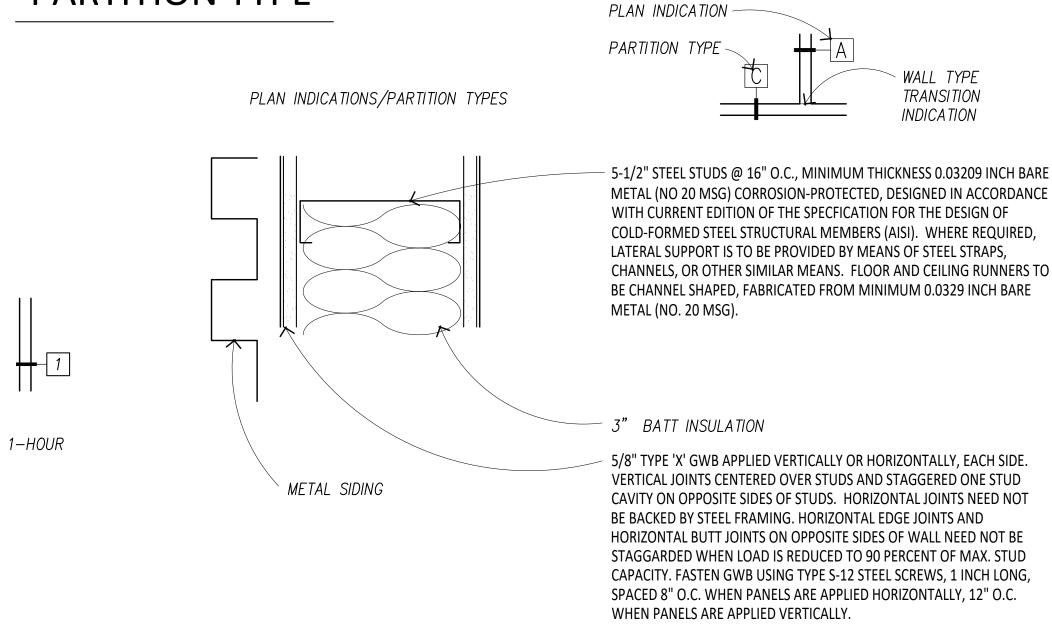
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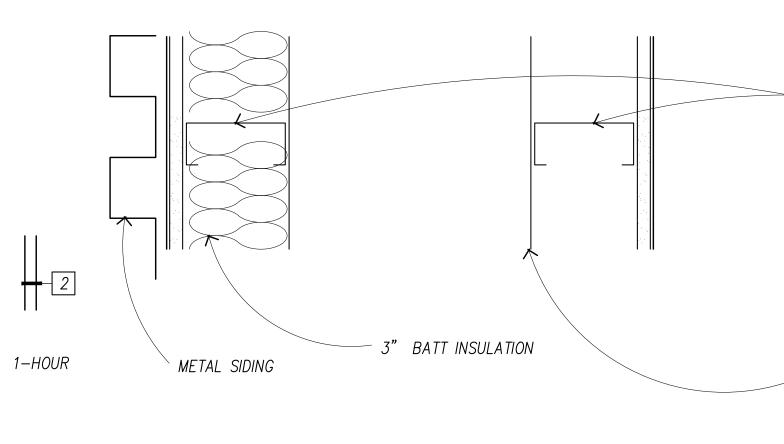
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PARTITION SCHEDULE

PARTITION TYPE



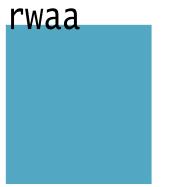


- 3-5/8" STEEL STUDS @ 16" O.C., MINIMUM THICKNESS 0.03209 INCH BARE METAL (NO 20 MSG) CORROSION-PROTECTED, DESIGNED IN ACCORDANCE WITH CURRENT EDITION OF THE SPECFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS (AISI). WHERE REQUIRED, LATERAL SUPPORT IS TO BE PROVIDED BY MEANS OF STEEL STRAPS, CHANNELS, OR OTHER SIMILAR MEANS. FLOOR AND CEILING RUNNERS TO BE CHANNEL SHAPED, FABRICATED FROM MINIMUM 0.0329 INCH BARE METAL (NO. 20 MSG).

5/8" TYPE 'X' GWB APPLIED VERTICALLY OR HORIZONTALLY, EACH SIDE. VERTICAL JOINTS CENTERED OVER STUDS AND STAGGERED ONE STUD CAVITY ON OPPOSITE SIDES OF STUDS. HORIZONTAL JOINTS NEED NOT BE BACKED BY STEEL FRAMING. HORIZONTAL EDGE JOINTS AND HORIZONTAL BUTT JOINTS ON OPPOSITE SIDES OF WALL NEED NOT BE STAGGARDED WHEN LOAD IS REDUCED TO 90 PERCENT OF MAX. STUD CAPACITY. FASTEN GWB USING TYPE S-12 STEEL SCREWS, 1 INCH LONG, SPACED 8" O.C. WHEN PANELS ARE APPLIED HORIZONTALLY, 12" O.C. WHEN PANELS ARE APPLIED VERTICALLY.

DOOR SCHEDULE

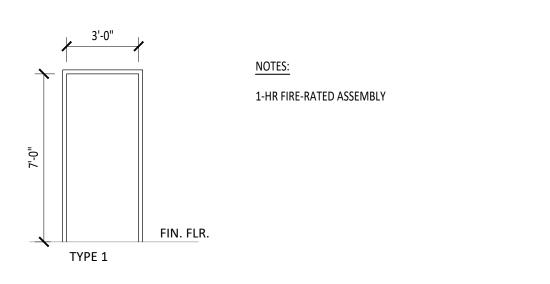
| MARK | OPENING SIZE | ROOM | DOOR TYPE | FRAME TYPE (HM) | THICKNESS | DOOR CONST. | FACING & FINISH | GLASS | RATING | HARDWARE GROUP | COMMENTS |
|------|---------------|--------------|--------------|-----------------------|-----------|----------------|--------------------|-------|--------|-------------------|--------------------|
| 001 | 3'-0" X 7'-0" | STAIR ACCESS | А | 1 | 1-3/4" | НМ | TYP. | - | 1HR | - | SEE SPECIFICATIONS |
| | | | | | | | | | | | |



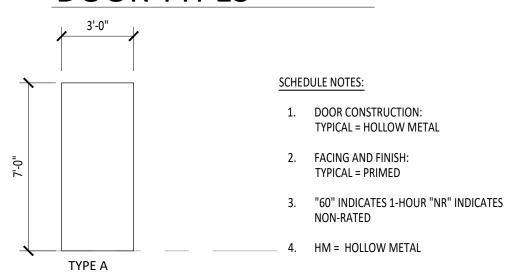
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HOLLOW METAL FRAME SCHEDULE



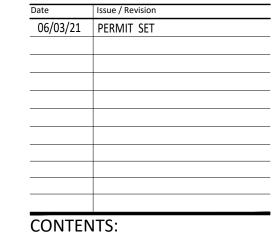
DOOR TYPES



PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



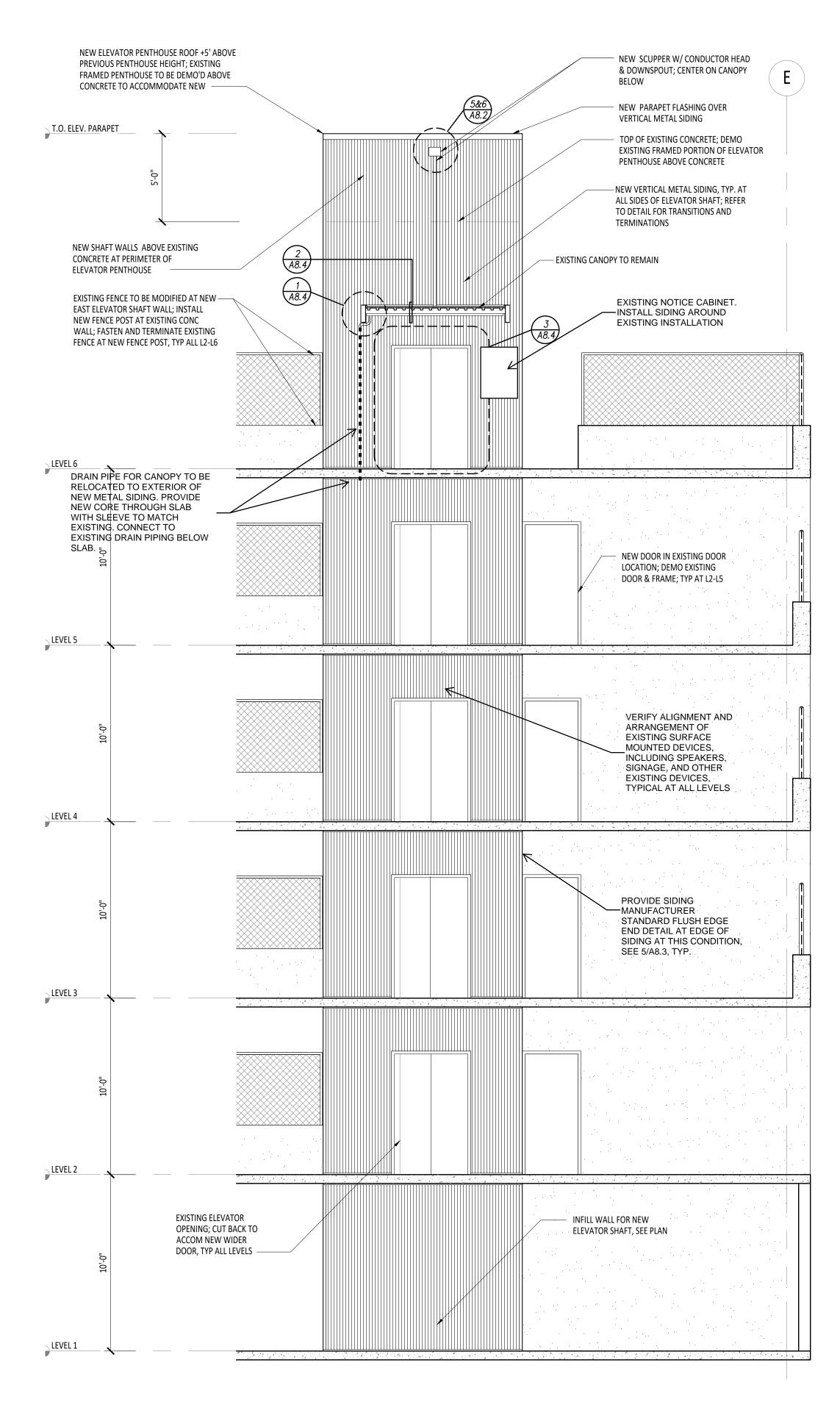


DOOR & PARTITION SCHEDULES

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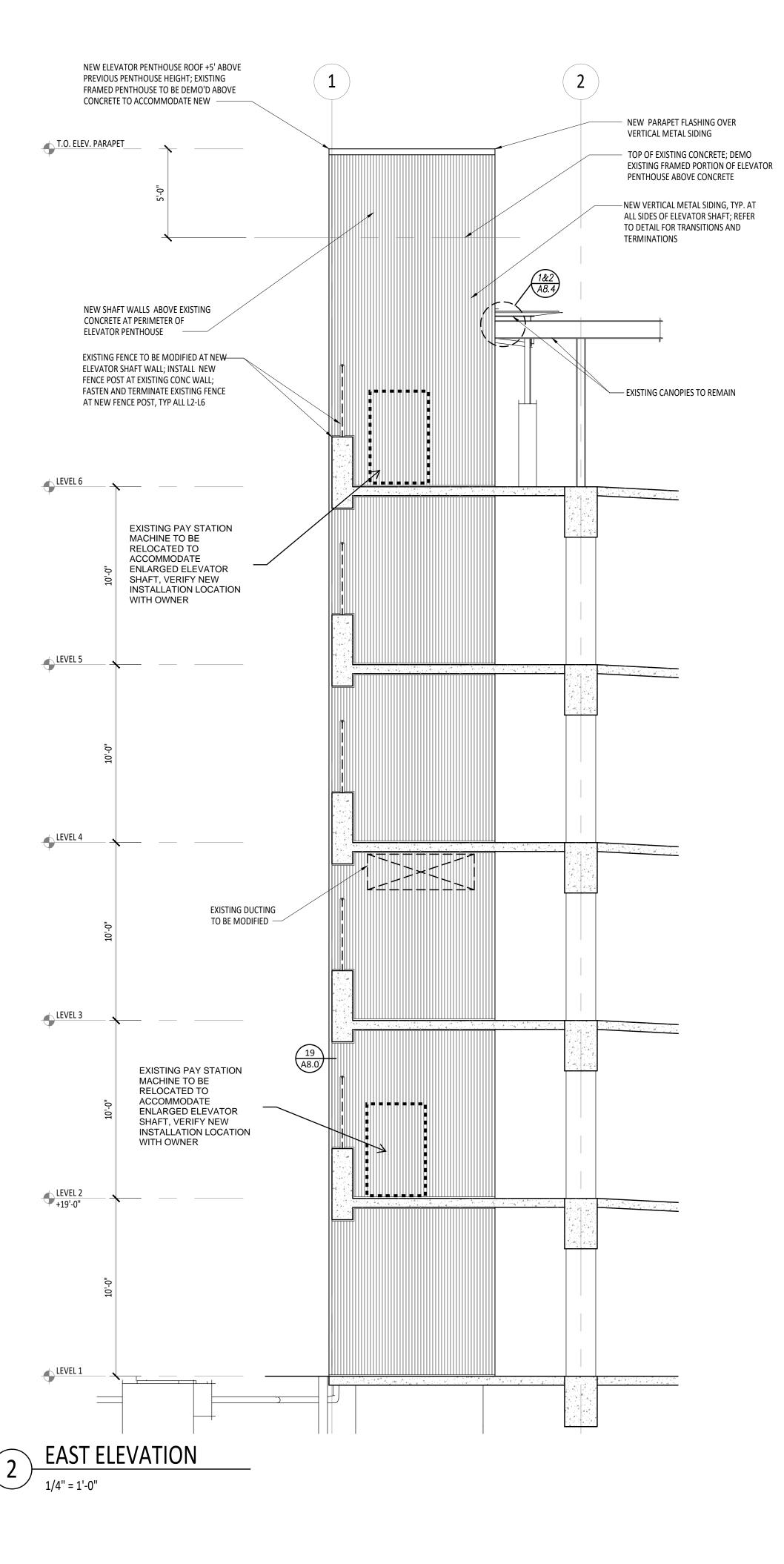
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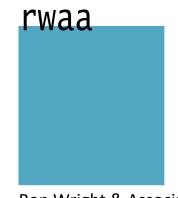
A2.6



NORTH ELEVATION

1/4" = 1'-0"





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PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



| Date | Issue / Revision | | | | | |
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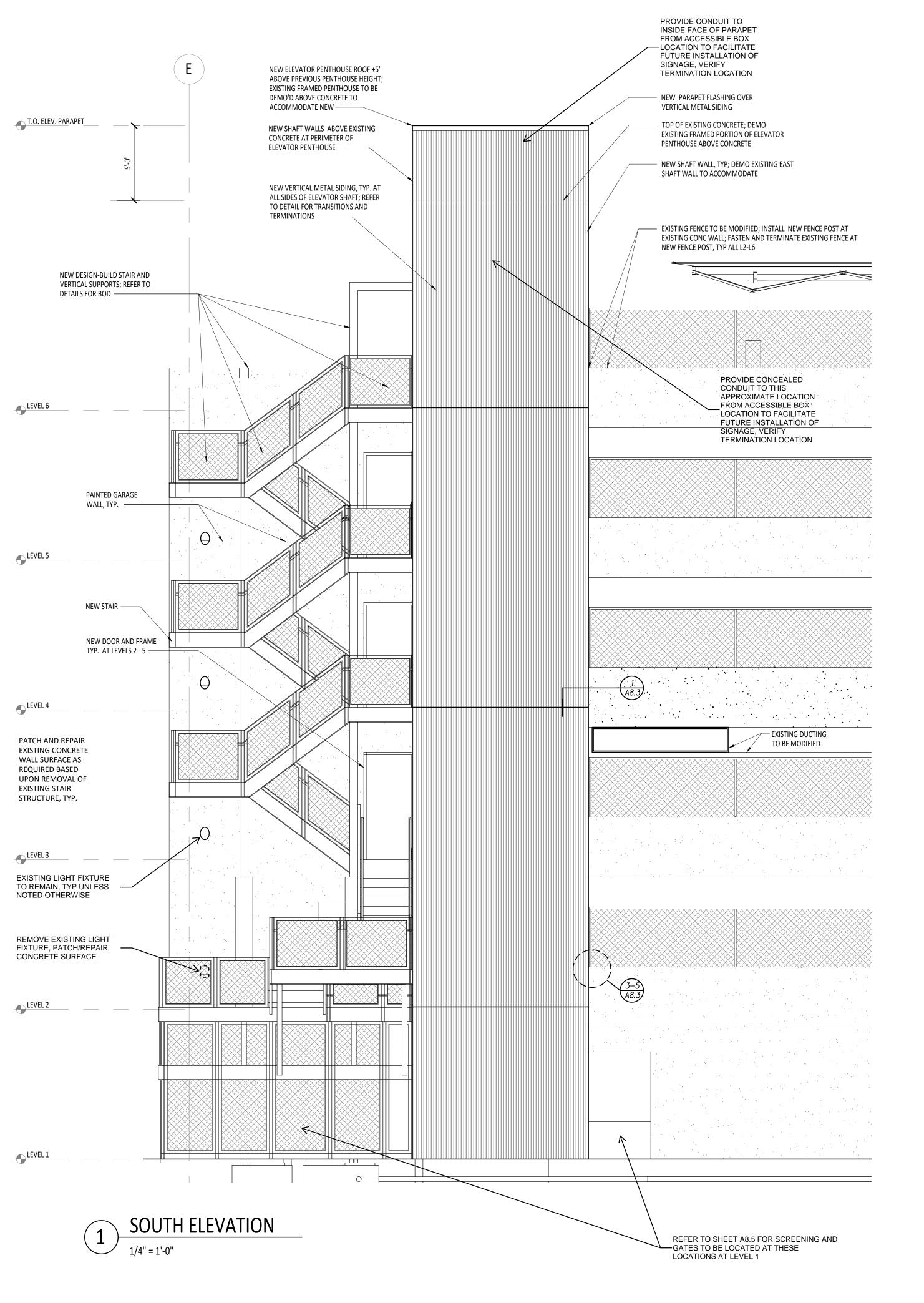
ELEVATIONS

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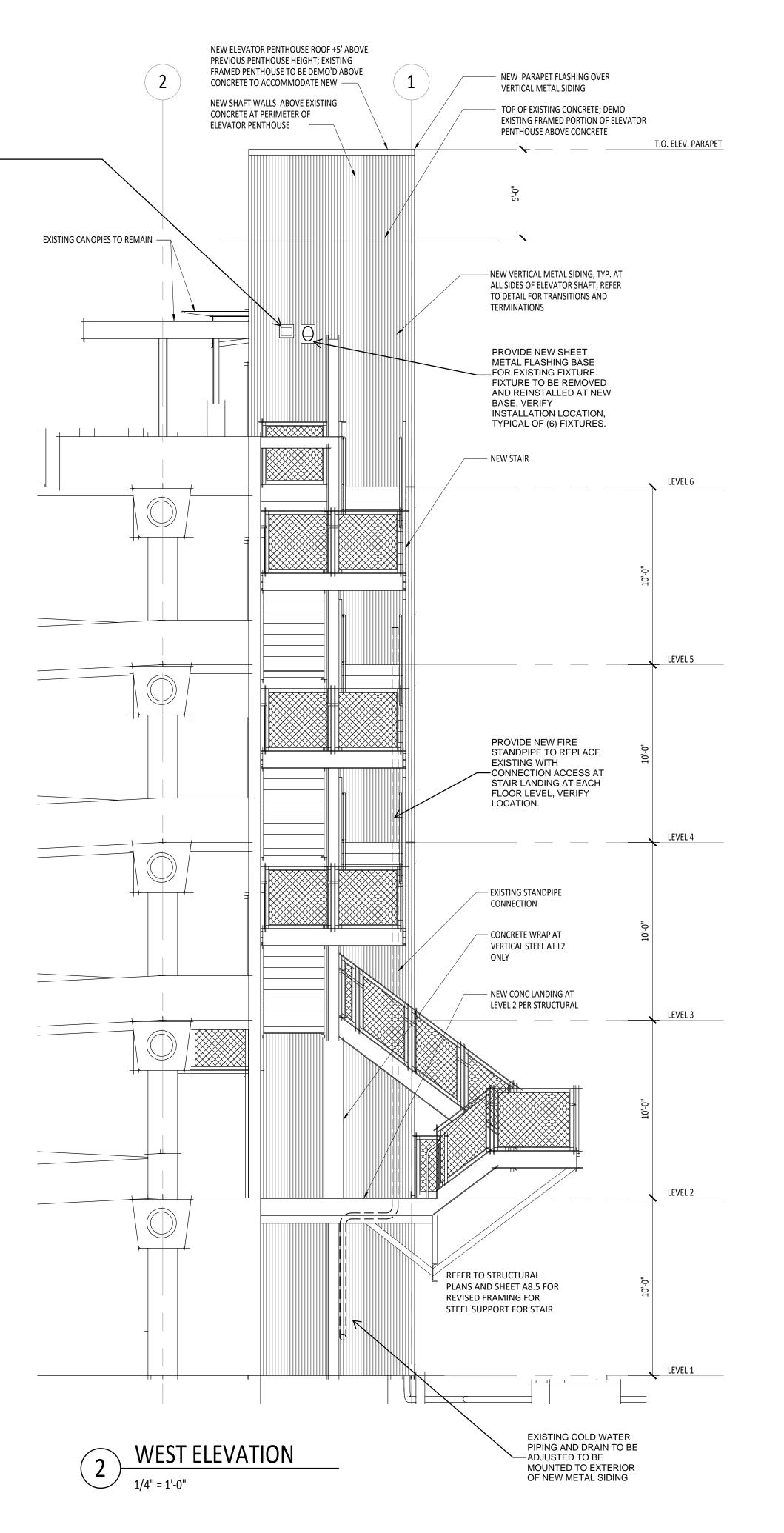
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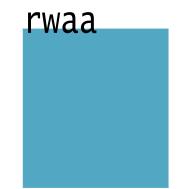
FILE:
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A3.0



REMOVE EXISTING BATTERY PACK
EMERGENCY EGRESS LIGHT FIXTURES.
INSTALL NEW FIXTURES AT METAL SIDING
WITH SHEET METAL FLASHING BASE,
TYPICAL OF (6) LOCATIONS. NEW FIXTURE:
HUBBELL LIGHTING PGW 782520862060, PG
SERIES, COLOR - WHITE, NUMBER OF LAMPS:
4, LAMP TYPE: SOLID STATE HIGH OUTPUT
LEDS, VOLTAGE RATING: 120/277 VAC.





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PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101

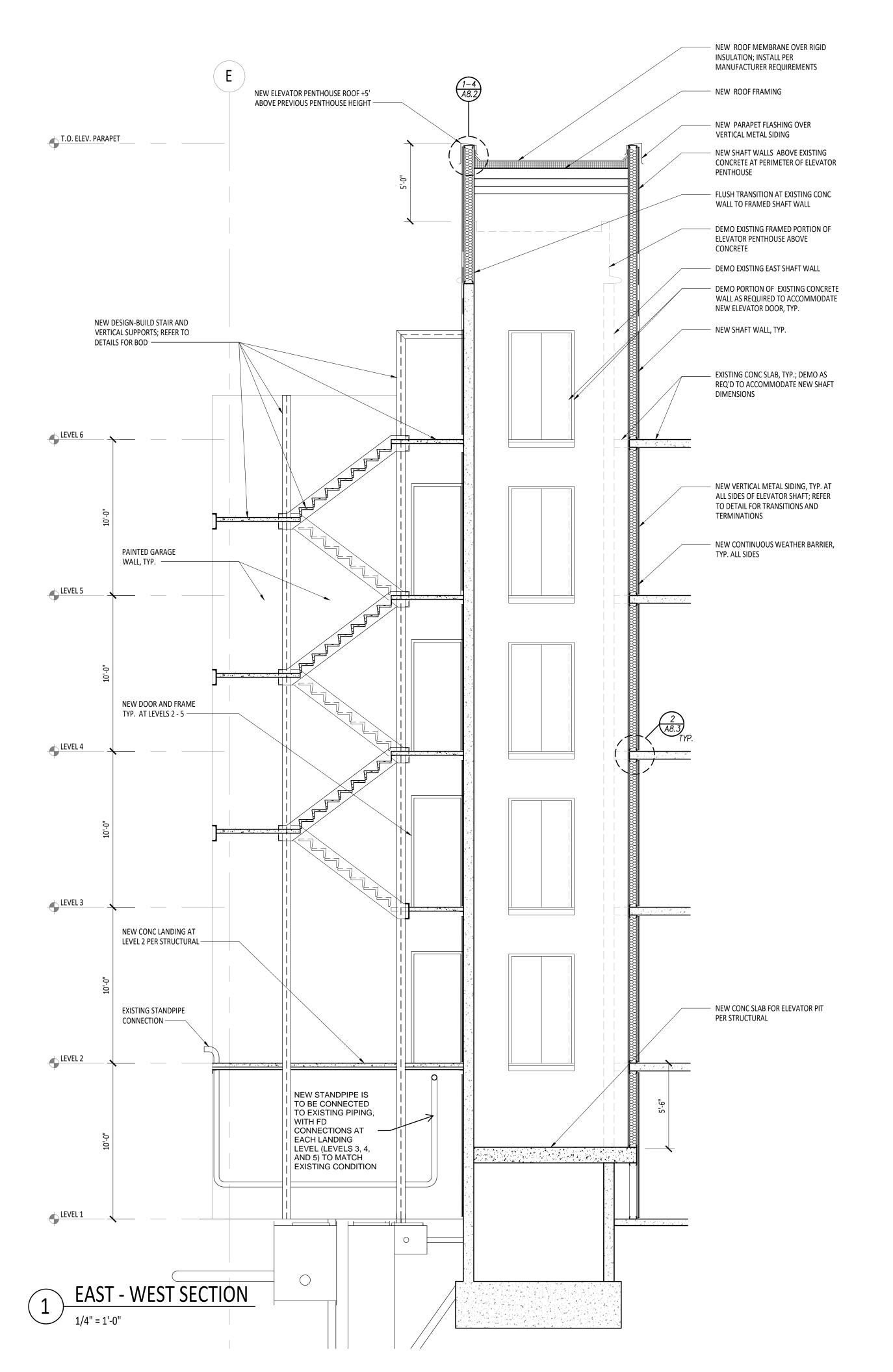


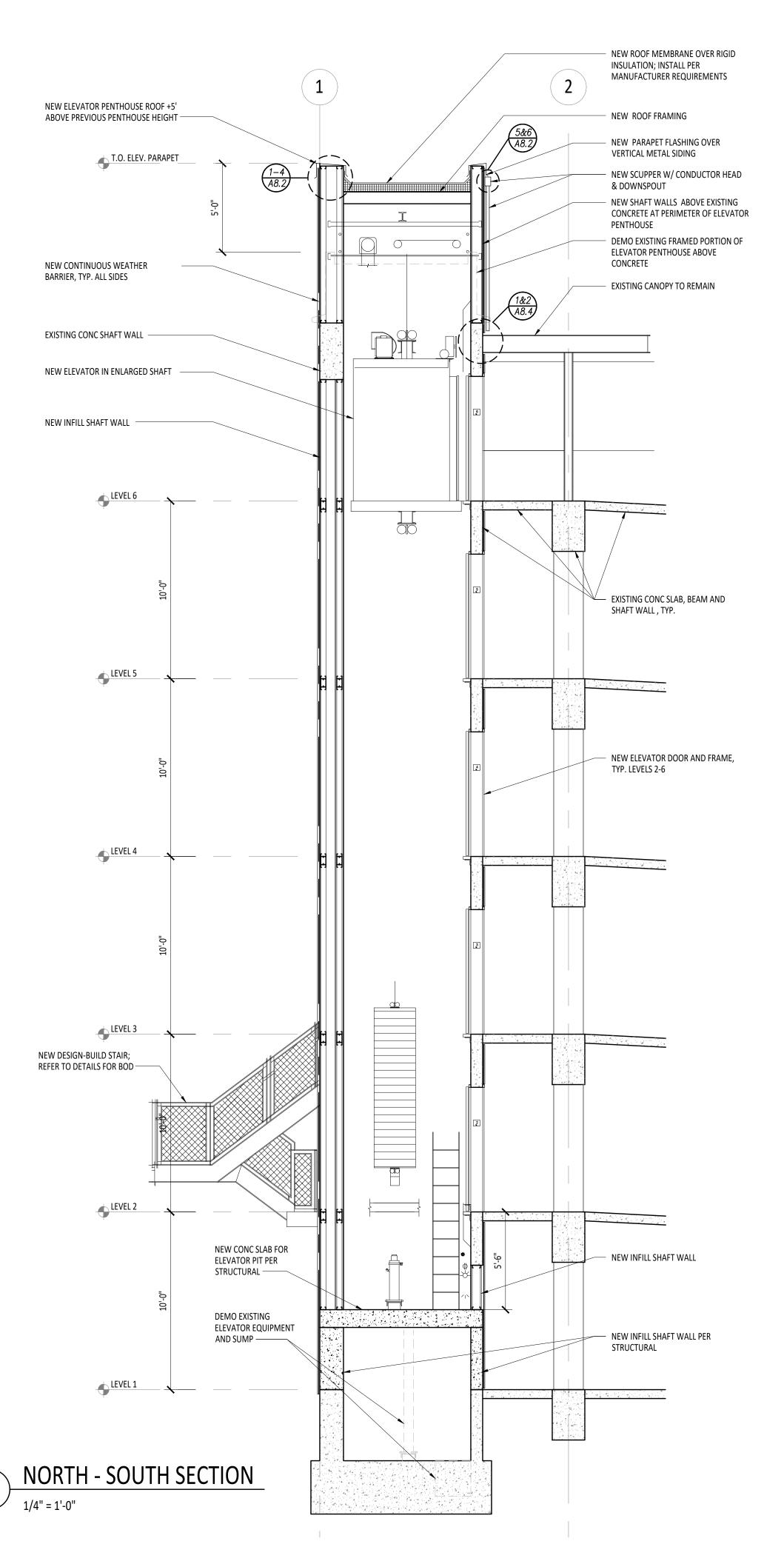
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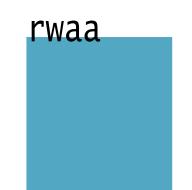
ELEVATIONS

PROJECT NO.: 20020
FILE:
SHEET:

A3.1







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PC-1S ELEVATOR AND STAIR REPLACEMENT

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SECTIONS

SCALE:

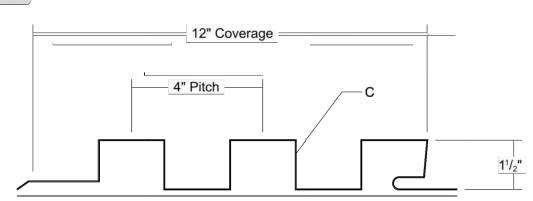
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TL-1222 PANEL

Condense Technica Reference

WALL PANEL



ARCHITECTURAL COMMERCIAL INDUSTRIAL PANEL

CONCEALED FASTENED

12" COVERAGE

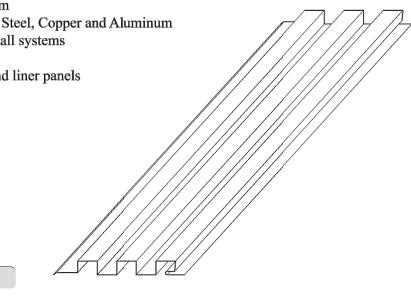
SOFFIT, FASCIA, WALL AND LINER PANEL

OPEN FRAMING OR SOLID SUBSTRATE

PANEL OVERVIEW

- ► Finish: Standard: PVDF
- Optional: multi-pass Kynar 500[®], Marblique, Plastisol, Polyester and MS Colorfast45[®]

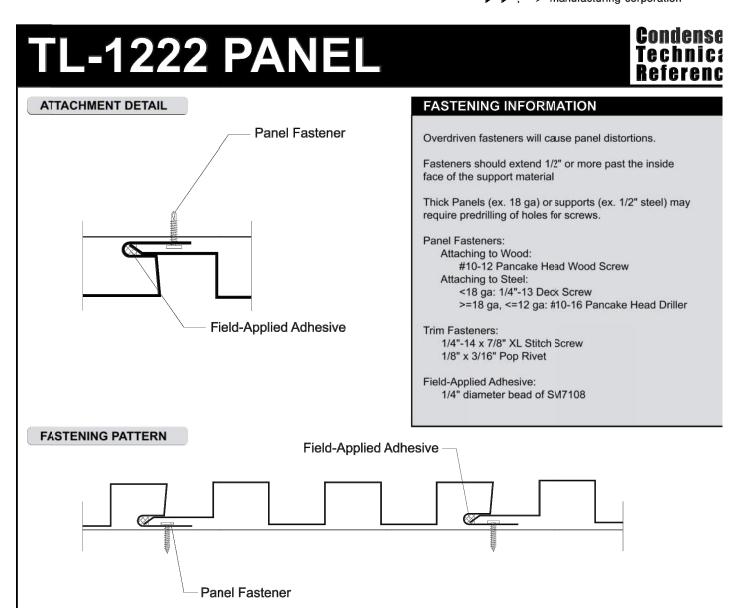
 Corrosion Protection: AZ50 per ASTM A 792 for painted Galvalume[®] G90 per ASTM A 653 for Galvanized
- ► Gauges: 24 ga, 22 ga, 20 ga and 18 ga are standard; 0.032" and 0.040" aluminum are optional
- ▶ 12" panel coverage, 1½" panel height
- Crisp 90° vertical box ribs on 4" centers, concealed fastened, non-end lapping panel system
- ▶ Panel Length: 5' minimum, 30' maximum
- ▶ Optional material availability: Stainless Steel, Copper and Aluminum
- ► Use on single-skin or field-assembled wall systems
- Custom capabilities include: -Perforated panels for wind screens and liner panels



TESTING AND APPROVALS

- ▶ UL 263 Fire Resistance Rating per assembly ► ASTM E 330 Uniform Static Air Pressure Difference
- ► ASTM E 1592 Uniform Static Air Pressure Difference

metal sales manufacturing corporation



| | SECTION PROPERTIES | | | | | | | | | LOV For | | | | | | | | | î | | | | | | | | |
|----|--------------------|--------------|---------------|--------|-----------|-------------|------------|--------|-------------|------------|--------|--------|--------------|--------|--------|----|----|----|----|----|----|----|----|----|----|----|----|
| | 185 141 | \c | | | mpression | Bottom In C | ompression | | Inward Load | | | | Outward Load | | | | | | | | | | | | | | |
| Ga | Width | Yield ksi | Weight psf | lxx | Sxx | lxx | Sxx | | | | | | Outward Load | | | | | | | | | | | | | | |
| | "' | Koi | poi | in⁴/ft | in³/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in⁴/ft | in³/ft | 2' | 3' | 4' | 5' | 6' | 8' | 2' | 3' | 4' | 5' | 6' | 8' |
| 24 | 12 | 50 | 1.74 | 0.1448 | 0.1609 | 0.1392 | 0.1593 | 50 | 45 | 39 | 34 | 28 | 17 | 50 | 45 | 41 | 36 | 31 | 22 | | | | | | | | |
| 22 | 12 | 50 | 2.27 | 0.2036 | 0.2339 | 0.1934 | 0.2304 | 50 | 45 | 39 | 34 | 28 | 17 | 50 | 45 | 41 | 36 | 31 | 22 | | | | | | | | |
| 20 | 12 | 33 | 2.77 | 0.2760 | 0.3340 | 0.2580 | 0.3266 | 50 | 45 | 39 | 34 | 28 | 17 | 50 | 45 | 41 | 36 | 31 | 22 | | | | | | | | |
| 18 | 12 | 33 | 3.64 | 0.3720 | 0.4610 | 0.3500 | 0.4497 | 50 | 45 | 39 | 34 | 28 | 17 | 50 | 45 | 41 | 36 | 31 | 22 | | | | | | | | |

- Theoretical section properties have been calculated per AISI 2016 'North American Specification for the Design of Cold-Formed Steel Structural Members'. Ixx and Sxx are effective section properties for deflection and bending. Allowable load is calculated in accordance with AISI 2016 specifications considering bending, shear, combined bending and shear and deflection. Allowable load considers the 3 or more equal spans condition. Allowable load does not address web crippling, fasteners, support material or load testing. Panel weight is not considered.
- Deflection consideration is limited by a maximum deflection ratio of L/180 of span. Allowable loads do not include a 1/3 stress increase for wind.

773 metal sales metalsales.us.com

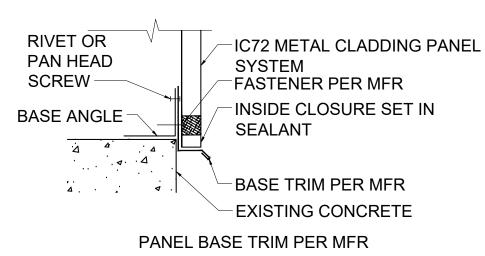
Jacksonville, FL 800.394.4419 Jefferson, OH 800.321.5833

Seattle, WA 800.431.3470 Sellersburg, IN 800.999.777

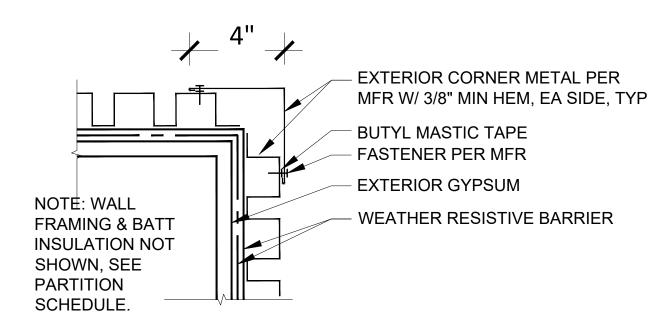




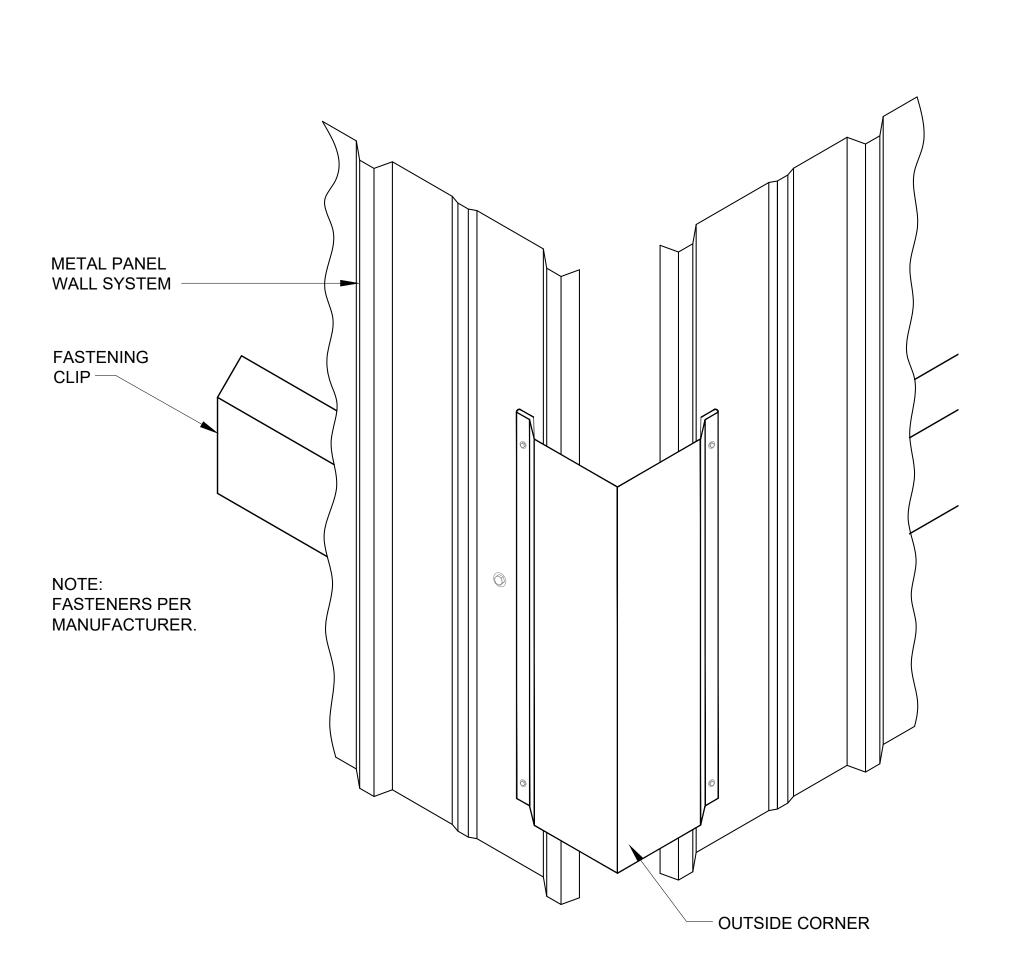








METAL SIDING EXTERIOR 5 CORNER- PLAN/SECTION



METAL SIDING AT EXTERIOR WALL CORNER - AXON

PC-1S **ELEVATOR** AND STAIR **REPLACEMENT**

rwaa

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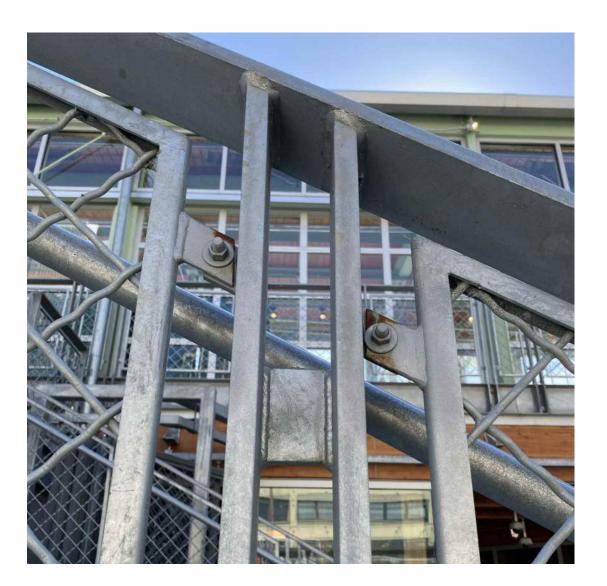
PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101

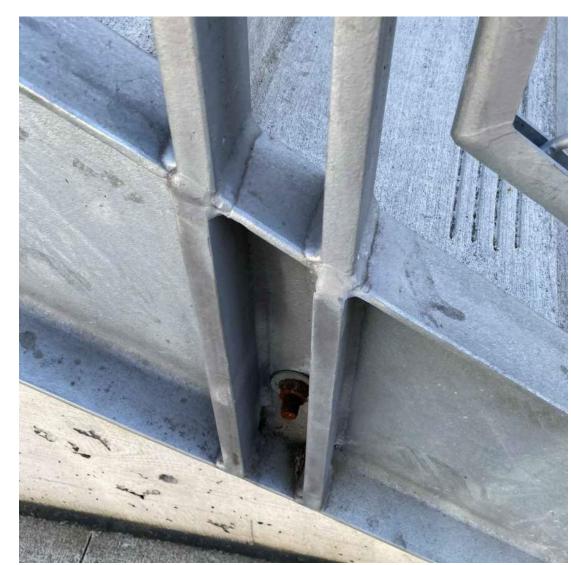


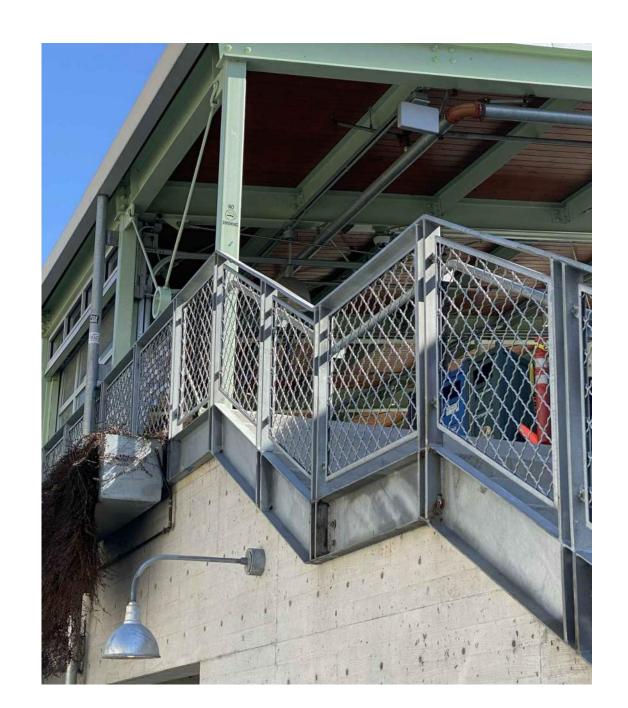
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DETAILS

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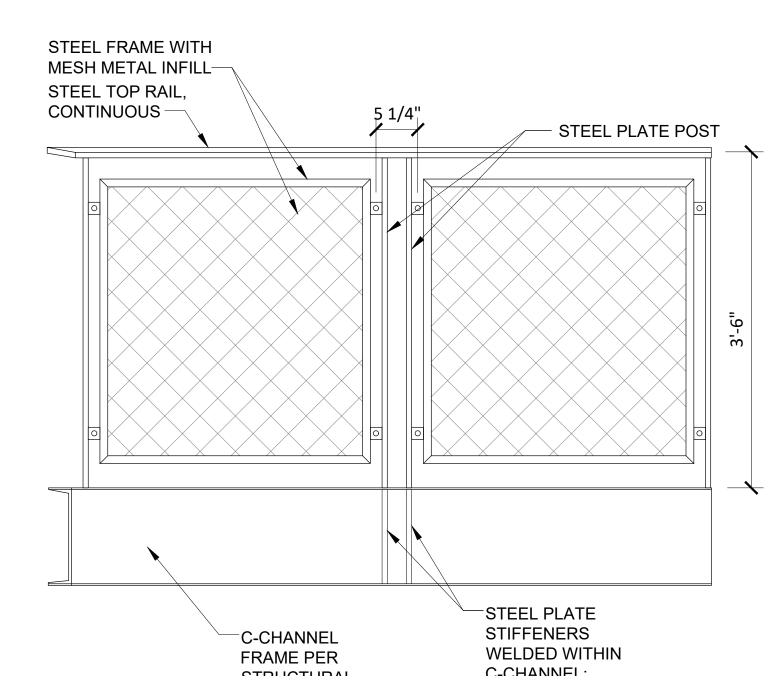




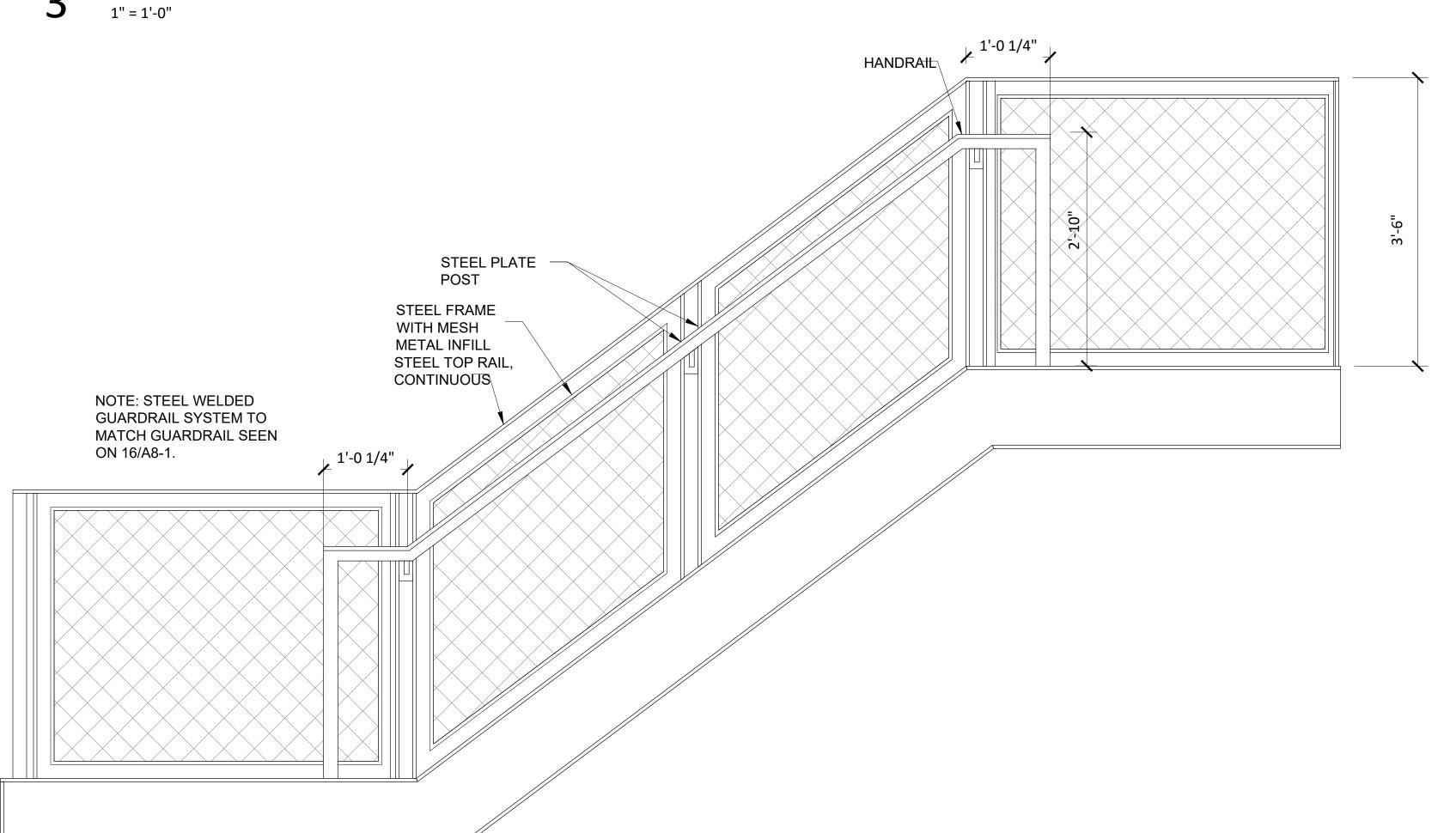


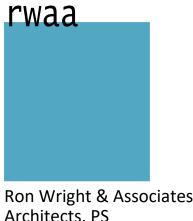
EXISTING SIDING SAMPLE FROM PIKE

16 MARKET TO BE MATCHED - PHOTOS



NEW GUARDRAIL AT STAIR - ELEVATION





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PC-1S ELEVATOR AND STAIR REPLACEMENT

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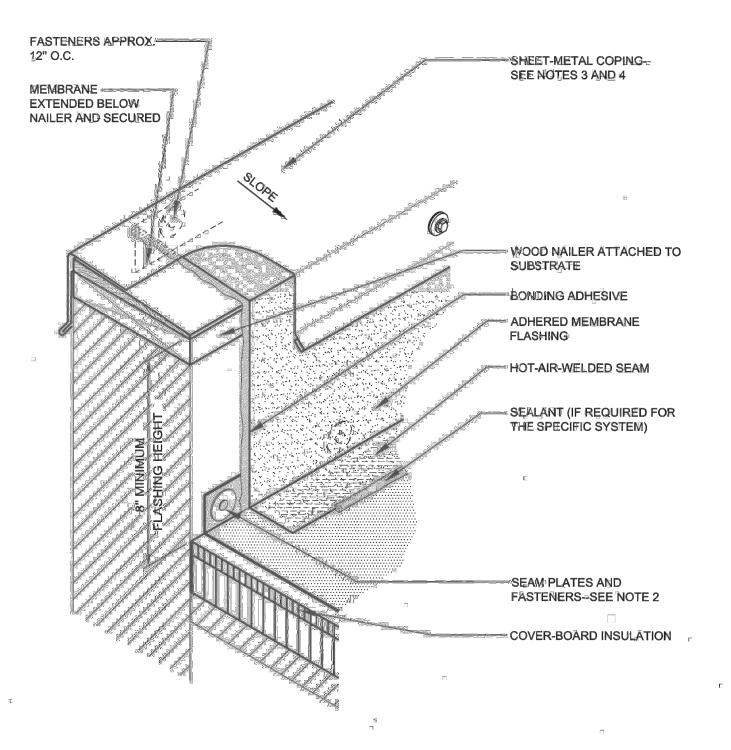
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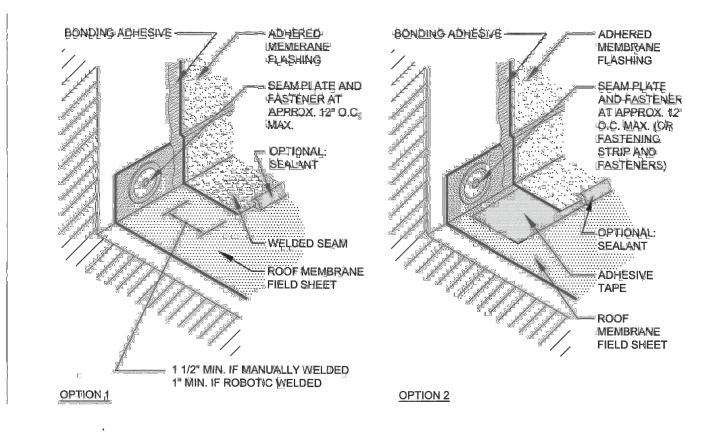
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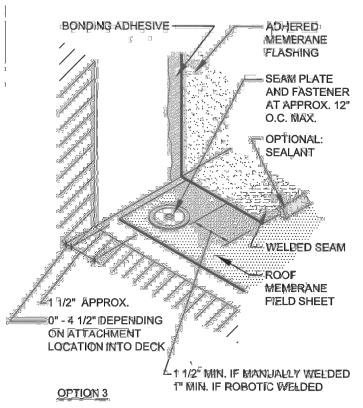


- 1. THIS DETAIL DOES NOT ALLOW FOR DIFFERENTIAL MOVEMENT BETWEEN THE DECK AND WALL. SEE DETAIL SP-7 FOR EXPANSION JOINT
- 2. REFER TO SECTION 10.1-INFORMATION APPLICABLE TO ALL CONSTRUCTION DETAILS FOR ALTERNATIVE BASE SECUREMENT OPTIONS.
- 3. REFER TO THE ARCHITECTURAL METAL FLASHING SECTION OF THE NRCA ROOFING MANUAL: ARCHITECTURAL METAL FLASHING, CONDENSATION CONTROL AND REROOFING -2010 FOR DESIGN, JOINERY AND SEGUREMENT OPTIONS FOR COPINGS,

BASE FLASHING AT PARAPET WALL WITH METAL COPING

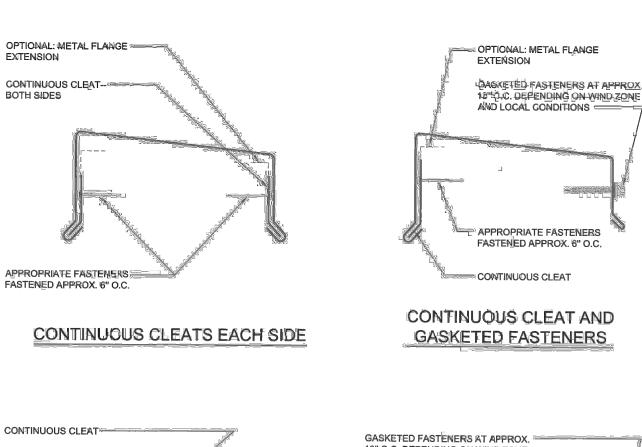
NTS

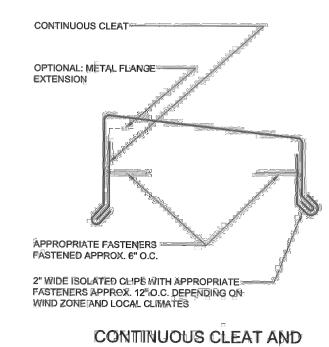


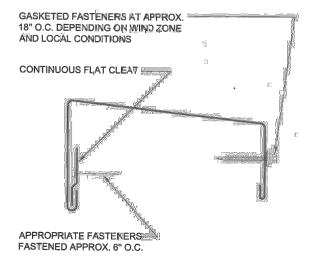


BASE SECUREMENT OPTIONS FOR

4 SINGLE PLY MEMBRANE







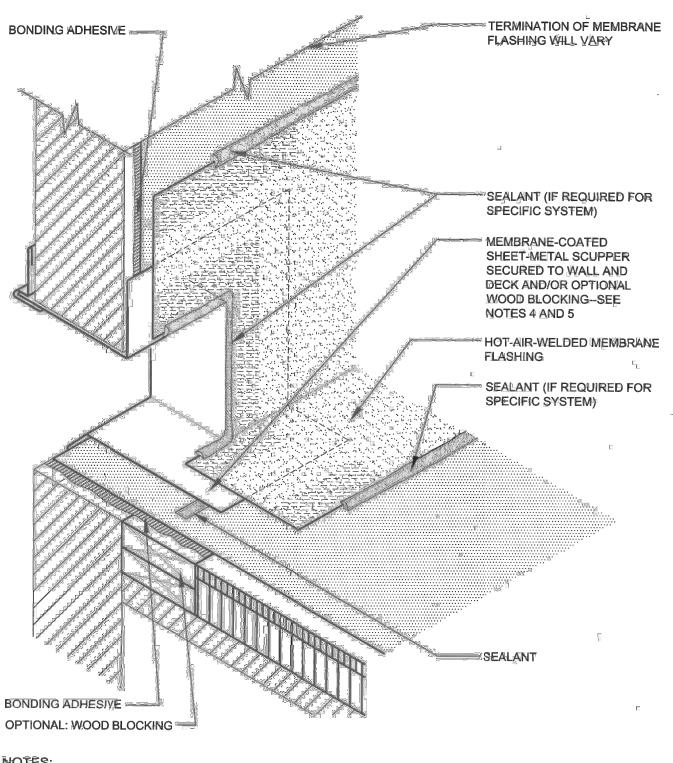
CONTINUOUS CLEAT WITH FLAT DRIP AND GASKETED **FASTENERS**

NOTES: WOOD NAILERS, SHIMS AND WALL CONSTRUCTION NOT SHOWN FOR CLARITY.

ISOLATED CLIPS

REFER TO SMAFFOR GUIDELINES ON MINIMUM GAUGES AND CLEAT/CLIP INFORMATION AND SMAFTO SMAFFOR JOINERY OPTIONS. CONSIDERATION MUST BE GIVEN TO WIND ZONE AND LOCAL CONDITIONS FOR THE SELECTION OF METAL GAUGE, PROFILE AND FASTENER SCHEDULE. SEVERE CONDITIONS OR CODE AND REGULATORY BODIES MAY REQUIRE MORE CONSERVATIVE DESIGNS. WHEN USING THE ABOVE INFORMATION, ADDITIONAL ITEMS SHOULD BE CONSIDERED, SUCH AS FASTENING PATTERNS. 4. REFER TO SECTION 4.1-INFORMATION APPLICABLE TO ALL CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.

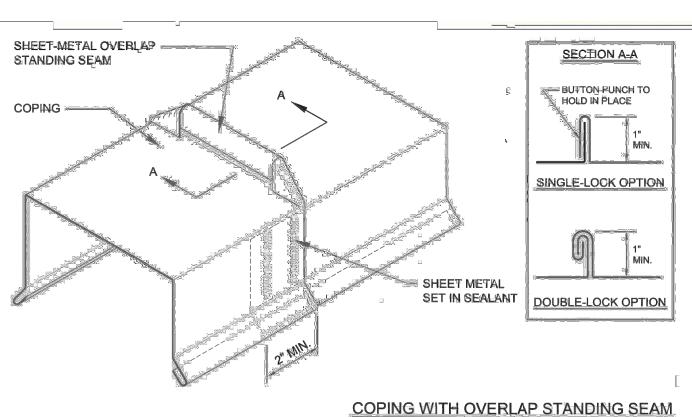
2 COPING SECUREMENT OPTIONS

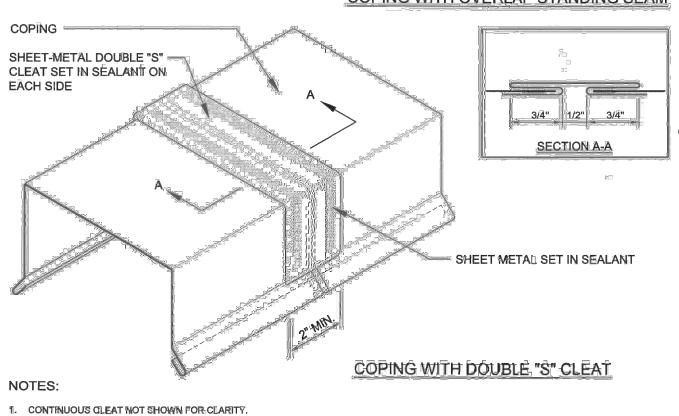


- THIS DETAIL DOES NOT ALLOW FOR DIFFERENTIAL MOVEMENT BETWEEN THE DECK AND WALL CONDUCTOR HEAD TO BE FINCH MINIMUM BELOW BOTTOW OF THROUGH-WALL SCUPPER.
- REFER TO SECTION 10.17 INFORMATION APPLICABLE TO ALL CONSTRUCTION DETAILS FOR ALTERNATIVE BASE SECUREMENT OPTIONS. REFER TO THE ARCHITECTURAL METAL FLASHING SECTION OF THE NRCA ROOFING MANUAL: ARCHITECTURAL METAL FLASHING, CONDENSATION CONTROL AND REROOFING 2010 FOR DESIGN, JOINERY AND SECUREMENT OFTIONS FOR SCUPPERS: REFER TO SECTION 10.1-INFORMATION APPLICABLE TO ILL CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.

BASE FLASHING AT THROUGH-WALL

5 SCUPPER





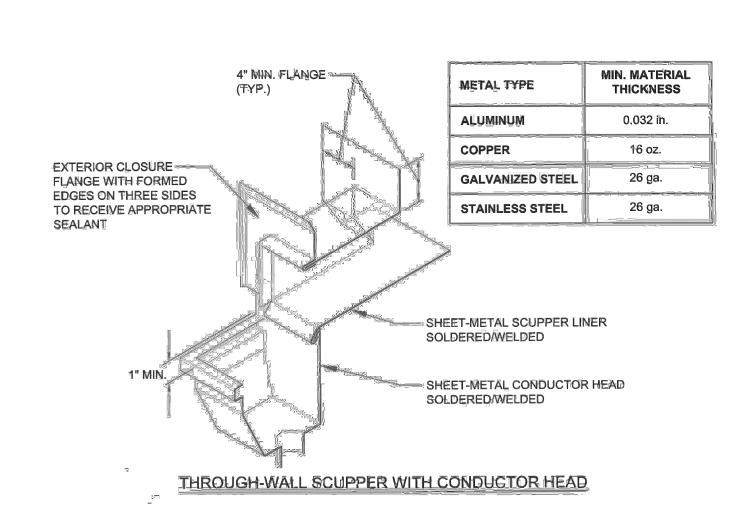
REFER TO SECTION 4.1-INFORMATION APPLICABLE TO ALL CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.

REMOVAL OF JUNDERLYING METAL HEM MAY HELP PACILITATE PROPER NESTING.

COPING JOINERY IS SIMILAR, ON BOTH SIDES.

REFER TO SM-3 FOR SECUREMENT OPTIONS AND SM-4 FOR GUIDELINES ON MINIMUM-GAUGES AND CLEAT/CLIP INFORMATION:

3 COPING JOINERY OPTIONS



THROUGH-WALL SCUPPER WITH 6 CONDUCTOR HEAD



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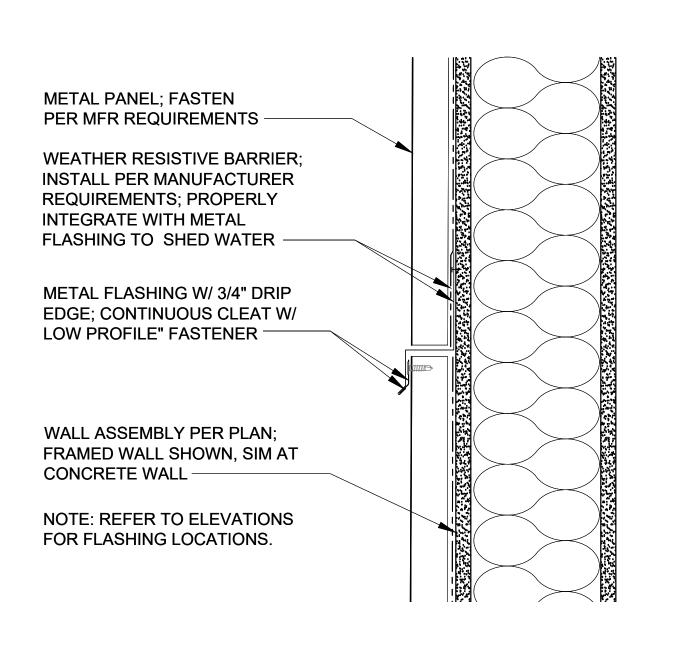
PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



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DETAILS

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THROUGH-WALL FLASHING

- BASE METAL BELOW, SET IN

METAL CORNER TRIM PER

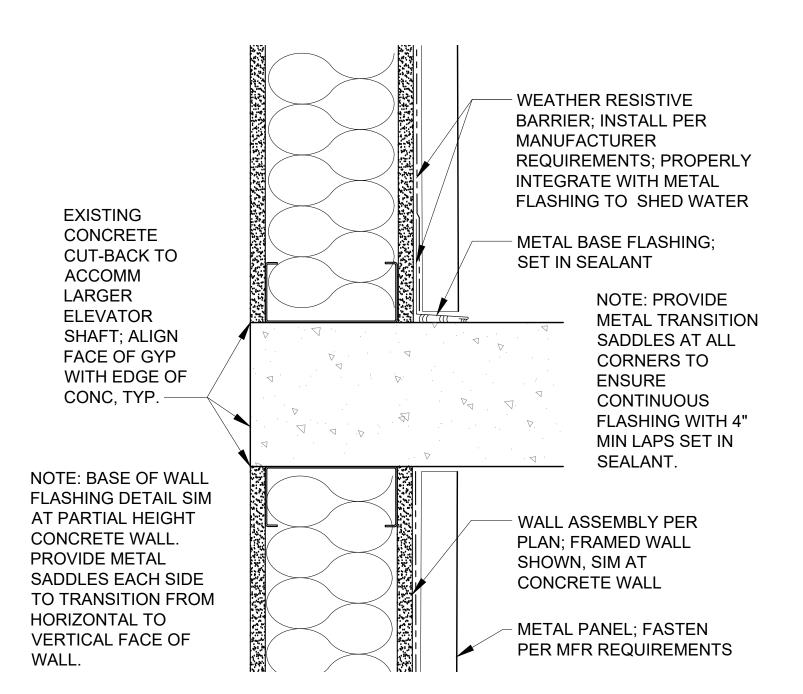
MANUFACTURER

HEIGHT CONCRETE WALL

SEALANT; SIM TO DETAIL 2/A8.3

SIDING TRANSITION AT PARTIAL

3" = 1'-0"



SIDING TERMINATIONS AT **CONCRETE SLAB**

3" = 1'-0"

EXISTING SHAFT WALL EXISTING PARTIAL HEIGHT CONC WALL BELOW FRAMED SHAFT WALL **NEW SHAFT WALLS PER** PLAN; REFER TO PARTITION SCHEDULE WEATHER RESISTIVE BARRIER; INSTALL PER MANUFACTURER REQUIREMENTS METAL PANEL; FASTEN PER MANUFACTURER REQUIREMENTS

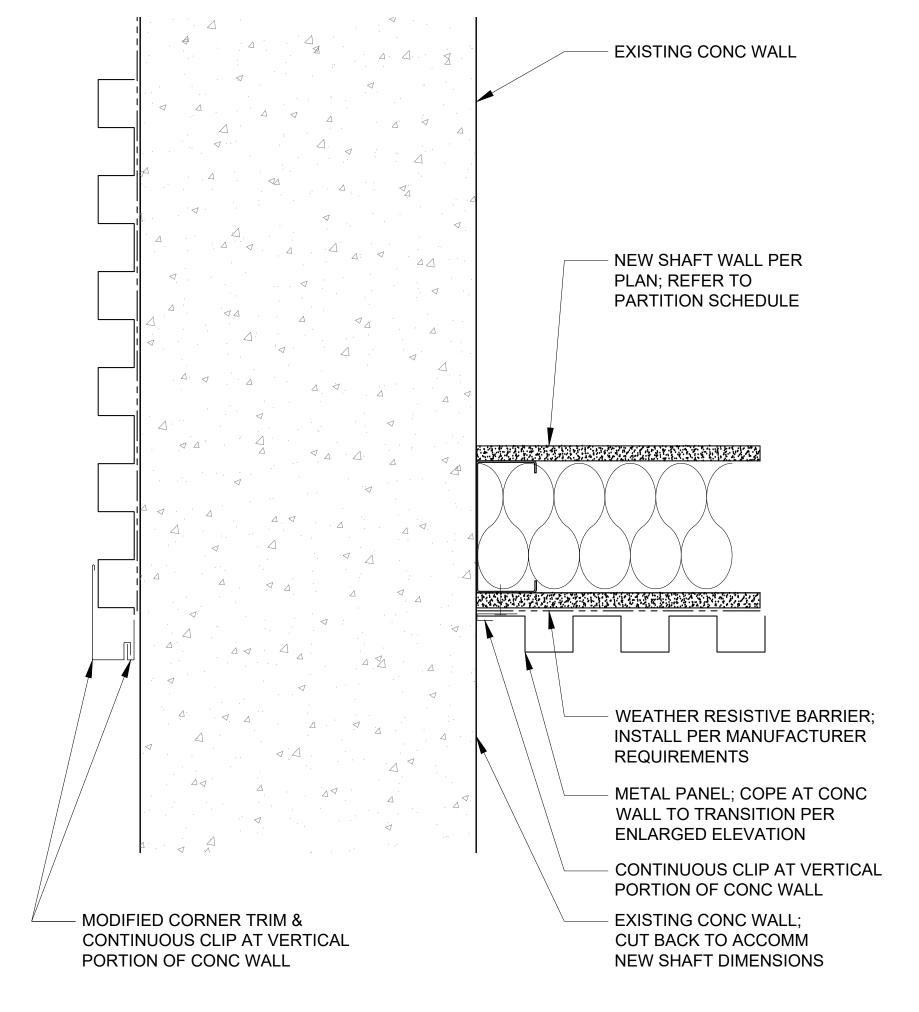
CONTINUOUS CLIP AT VERTICAL

PORTION OF CONC WALL

EXISTING CONC WALL;

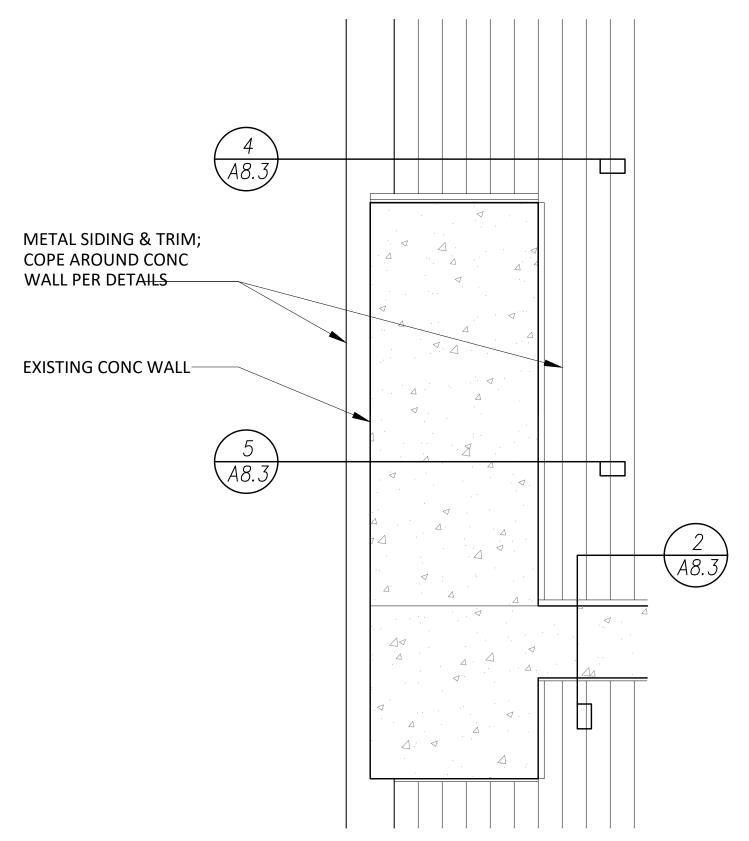
CUT BACK TO ACCOMM

NEW SHAFT DIMENSIONS



SIDING TRANSITION AT PARTIAL HEIGHT CONCRETE WALL

3" = 1'-0"



SIDING TERMINATIONS AT **CONCRETE SLAB**

1-1/2" = 1'-0"

PC-1S **ELEVATOR** AND STAIR **REPLACEMENT**

rwaa

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PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



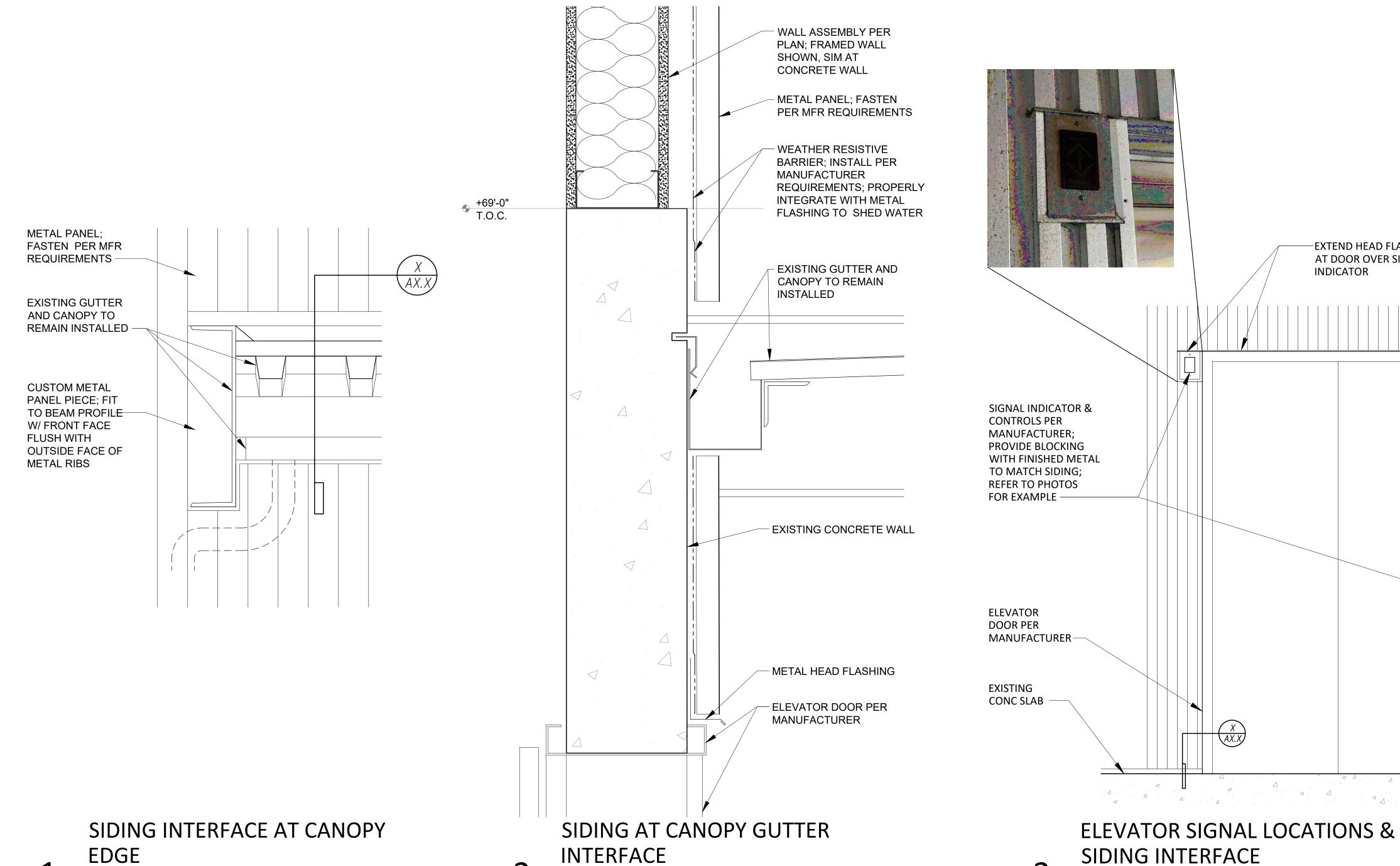
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DETAILS

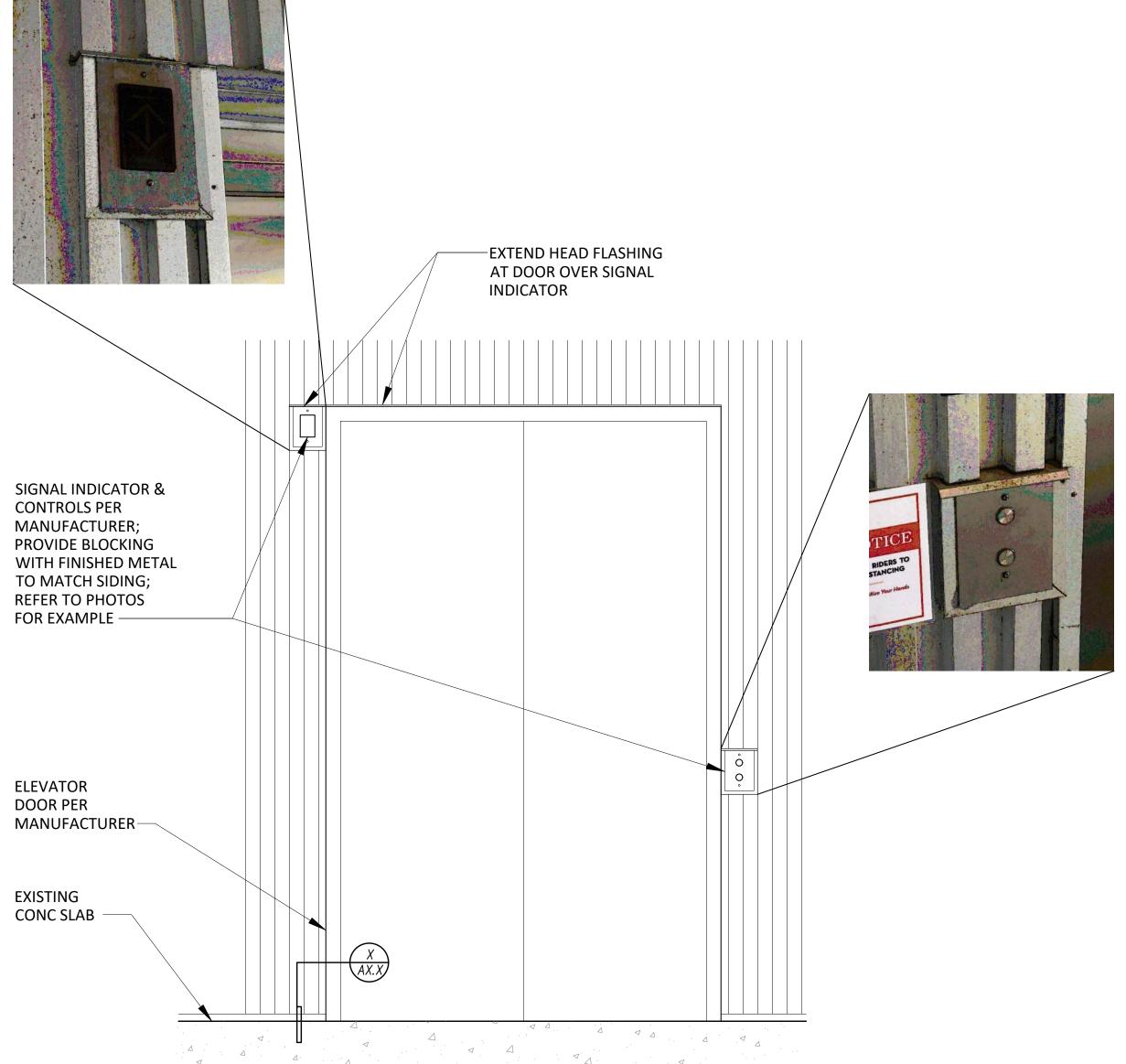
SCALE: AS NOTED PROJECT NO.: 20020 FILE: SHEET:



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3" = 1'-0"



1" = 1'-0"

PC-1S **ELEVATOR** AND STAIR **REPLACEMENT**

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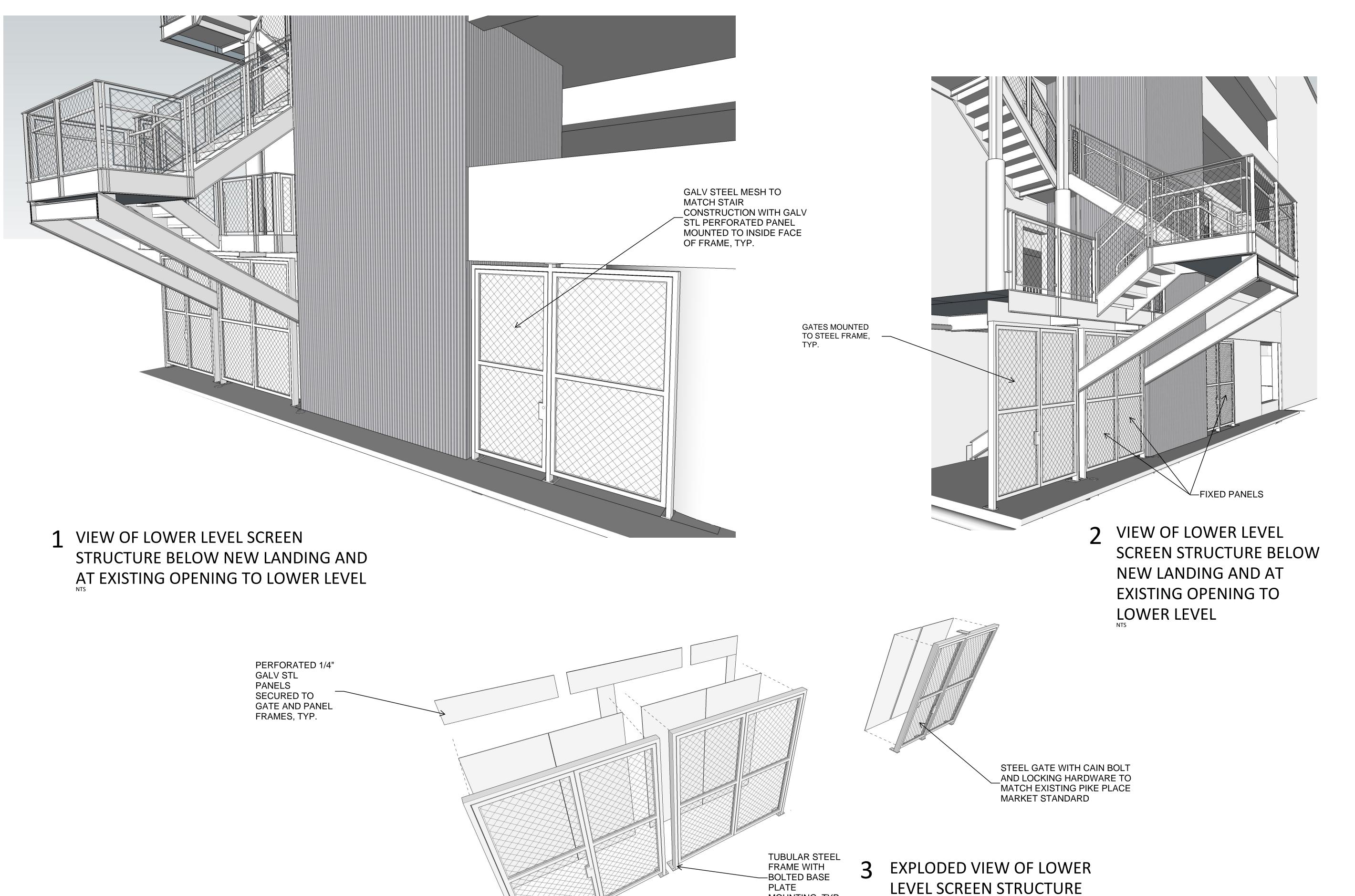
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DETAILS

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A8.4

EDGE



MOUNTING, TYP.

STEEL GATES WITH CAIN BOLT AND LOCKING HARDWARE TO MATCH EXISTING PIKE PLACE MARKET STANDARD BELOW NEW LANDING AND

AT EXISTING OPENING TO

LOWER LEVEL

rwaa

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PC-1S ELEVATOR AND STAIR REPLACEMENT

PIKE PLACE MARKET PDA 85 PIKE STREET, #500 SEATTLE, WA 98101



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THESE GENERAL NOTES ARE TO BE USED AS A SUPPLEMENT TO THE SPECIFICATIONS. ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, THE SPECIFICATIONS, THESE GENERAL NOTES AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ARCHITECT, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. ANY WORK DONE BY THE GENERAL CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE GENERAL CONTRACTOR'S RISK. THE GENERAL CONTRACTOR SHALL VERIFY AND COORDINATE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION. THE STRUCTURE HAS BEEN DESIGNED TO RESIST CODE SPECIFIED VERTICAL AND LATERAL FORCES AFTER THE CONSTRUCTION OF ALL STRUCTURAL ELEMENTS HAS BEEN COMPLETED. STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. THIS RESPONSIBILITY INCLUDES BUT IS NOT LIMITED TO

JOB SITE SAFETY; ERECTION MEANS, METHODS, AND SEQUENCES; TEMPORARY SHORING, FORMWORK, BRACING; USE OF EQUIPMENT AND CONSTRUCTION PROCEDURES. PROVIDE ADEQUATE RESISTANCE TO LOADS ON THE STRUCTURES DURING CONSTRUCTION PER SEI/ASCE STANDARD NO. 37-14 "DESIGN LOADS ON STRUCTURES DURING CONSTRUCTION."

CONSTRUCTION OBSERVATION BY THE STRUCTURAL ENGINEER IS FOR GENERAL CONFORMANCE WITH DESIGN ASPECTS ONLY AND IS NOT INTENDED IN ANY WAY TO REVIEW THE CONTRACTOR'S CONSTRUCTION PROCEDURES.

STANDARDS

ALL METHODS, MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED AND ADOPTED BY THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION.

CONTRACT DRAWINGS / DIMENSIONS

ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. CONSULTANT DRAWINGS BY OTHER DISCIPLINES ARE SUPPLEMENTARY TO ARCHITECTURAL DRAWINGS. REPORT DIMENSIONAL OMISSIONS OR DISCREPANCIES BETWEEN ARCHITECTURAL DRAWINGS AND STRUCTURAL, MECHANICAL, ELECTRICAL OR CIVIL DRAWINGS TO ARCHITECT PRIOR TO PROCEEDING WITH WORK.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS. PRIMARY STRUCTURAL ELEMENTS ARE DIMENSIONED ON STRUCTURAL PLANS AND DETAILS AND OVERALL LAYOUT OF STRUCTURAL PORTION OF WORK. SOME SECONDARY ELEMENTS ARE NOT DIMENSIONED, SUCH AS WALL CONFIGURATIONS, INCLUDING EXACT DOOR AND WINDOW LOCATIONS, ALCOVES, SLAB SLOPES AND DEPRESSIONS CURBS. ETC. VERTICAL DIMENSIONAL CONTROL IS DEFINED BY ARCHITECTURAL WALL SECTIONS AND BUILDING SECTIONS. STRUCTURAL DETAILS SHOW DIMENSIONAL RELATIONSHIPS TO CONTROL DIMENSIONS DEFINED BY ARCHITECTURAL DRAWINGS. DETAILING AND SHOP DRAWING PRODUCTION FOR STRUCTURAL ELEMENTS WILL REQUIRE DIMENSIONAL INFORMATION CONTAINED IN **BOTH** ARCHITECTURAL AND STRUCTURAL DRAWINGS.

DESIGN CRITERIA

VERTICAL LOADS

| AREA | DESIGN DEAD LOAD | LIVE LOAD (2) | PARTITION LOAD | CONCENTRATED LOADS | | | | | | | |
|----------|---------------------|---------------|-------------------|--------------------|--|--|--|--|--|--|--|
| STAIRS | 20 PSF | 100 PSF | | 300# | | | | | | | |
| LANDINGS | 50 PSF | 100 PSF | | 300# | | | | | | | |

SNOW: (MINIMUM ROOF SNOW LOAD = 25 PSF)

LATERAL FORCES

LATERAL FORCES ARE TRANSMITTED BY DIAPHRAGM ACTION OF LANDINGS TO EXISTING WALLS. LOADS ARE THEN TRANSFERRED TO FOUNDATION BY SHEAR WALL ACTION WHERE ULTIMATE DISPLACEMENT IS RESISTED BY PASSIVE PRESSURE OF EARTH AND/OR SLIDING FRICTION. OVERTURNING IS RESISTED BY DEAD LOAD OF THE STRUCTURE.

LATERAL FORCE RESISTING SYSTEM: ALL MEMBERS AND CONNECTIONS REFERRED TO AS LATERAL FORCE RESISTING SYSTEM (LFRS) SHALL COMPLY WITH REQUIREMENTS OF THE SEISMIC FORCE RESISTING SYSTEMM AND THE WIND FORCE RESISTING SYSTEM SET FORTH IN THE SPECIAL INSPECTION REQUIREMENTS OF IBC SECTION 1704 AND 1705, AND AS NOTED IN THE STATEMENT OF SPECIAL INSPECTIONS.

$$\begin{array}{c} \underline{\text{SEISMIC:}} \text{ (ASCE 7-16) V = CsW} \\ \\ \text{WHERE } \quad Cs = \frac{S_{DS}}{\left(\frac{R}{Ie}\right)} \; ; \; \text{WITH} \\ \\ \text{Cs MINIMUM} = 0.044 \; S_{DS} I_E \; \geq \; 0.01 \\ \text{OR} \\ \text{Cs MINIMUM} = \frac{0.5S_1}{\frac{R}{Ie}} \; \text{FOR S}_1 \; > \; 0.6g \\ \\ \text{Cs MAXIMUM} = \frac{S_{D1}}{T\left(\frac{R}{Ie}\right)} \; \text{FOR T} \; \leq \; T_L \\ \text{OR} \\ \text{Cs MAXIMUM} = \frac{S_{D1}T_L}{T^2\left(\frac{R}{L}\right)} \; \text{FOR T} \; > \; T_L \\ \end{array}$$

SEISMIC IMPORTANCE FACTOR, Ie = 1.0 RISK CATEGORY OF BUILDING PER IBC TABLE 1604.5 = II SPECTRAL RESPONSE ACCELERATIONS Ss = $1.4 \& S_1 = 0.488$ SITE CLASS PER TABLE 20.3-1 = D DESIGN SPECTRAL RESPONSE ACCELERATIONS $S_{DS} = 1.12 \& S_{D1} = 0.510$ SEISMIC DESIGN CATEGORY = D W = EFFECTIVE SEISMIC WEIGHT OF BUILDING = NA ANALYSIS PROCEDURE USED = EQUIVALENT LATERAL FORCE PROCEDURE RESPONSE MODIFICATION FACTOR PER TABLE 12.2-1, R = 5 Cs = 0.224

PIPES, DUCTS AND MECHANICAL EQUIPMENT SUPPORTED OR BRACED FROM STRUCTURE. CONFORM TO SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION, INC. PUBLICATION "SEISMIC RESTRAINT MANUAL: GUIDELINES FOR MECHANICAL SYSTEMS". SPRINKLER LINE ATTACHMENTS SHALL CONFORM TO NFPA PAMPHLET 13.

CONCRETE

CAST-IN-PLACE CONCRETE

DESIGN BASE SHEAR V = NA

MIX DESIGNS: THE CONTRACTOR SHALL DESIGN CONCRETE MIXES THAT MEET OR EXCEED THE REQUIREMENTS OF THE CONCRETE MIX TABLE. THE MIX DESIGNS SHALL FACILITATE ANTICIPATED PLACEMENT METHODS, WEATHER. REBAR CONGESTION, ARCHITECTURAL FINISHES, CONSTRUCTION SEQUENCING, STRUCTURAL DETAILS, AND ALL OTHER FACTORS REQUIRED TO PROVIDE A STRUCTURALLY SOUND, AESTHETICALLY ACCEPTABLE FINISHED PRODUCT. WATER REDUCING ADMIXTURES WILL LIKELY BE REQUIRED TO MEET THESE REQUIREMENTS. CONCRETE MIX DESIGNS SHALL CLEARLY INDICATE THE TARGET SLUMP. SLUMP TOLERANCE SHALL BE ± 1-1/2 INCHES.

AGGREGATE: COARSE AND FINE AGGREGATE SHALL CONFORM TO ASTM C33

CEMENT: CEMENT SHALL CONFORM TO ASTM C150, TYPE II PORTLAND CEMENT, UNLESS NOTED OTHERWISE.

FLYASH: SHALL CONFORM TO ASTM C618 CLASS C OR F, MAXIMUM LOSS OF IGNITION SHALL BE 1.0%.

SLAG: GROUND GRANULATED BLAST-FURNACE (GGBF) SLAG SHALL CONFORM TO ASTM C989 GRADE 100 OR 120.

ALTERNATE MIX DESIGNS: VARIATIONS TO THE MIX DESIGN PROPORTIONS MAY BE ACCEPTED IF SUBSTANTIATED IN ACCORDANCE WITH ACI 318, CHAPTER 19. PROVIDE SUBMITTALS A MINIMUM OF TWO WEEKS PRIOR TO BID FOR DETERMINATION OF ACCEPTABILITY.

ADMIXTURES: ADMIXTURES SHALL BE BY MASTER BUILDERS, W.R. GRACE, OR PRE-APPROVED EQUAL. ALL MANUFACTURER'S RECOMMENDATIONS SHALL BE FOLLOWED.

WATER: SHALL BE CLEAN AND POTABLE.

MAXIMUM CHLORIDE CONTENT: THE MAXIMUM WATER SOLUBLE CHLORIDE CONTENT SHALL NOT EXCEED 0.15% BY WEIGHT OF CEMENTITIOUS MATERIAL UNLESS NOTED OTHERWISE.

CONCRETE EXPOSED TO WEATHER: PROVIDE 5.0% TOTAL AIR CONTENT FOR ALL CONCRETE EXPOSED TO WEATHER. TOTAL AIR CONTENT IS THE SUM OF ENTRAINED AIR PROVIDED BY ADMIXTURES AND NATURALLY OCCURRING ENTRAPPED AIR. AIR CONTENT SHALL BE TESTED PRIOR TO BEING PLACED IN THE PUMP HOPPER OR BUCKET; IT IS NOT REQUIRED TO BE TESTED AT THE DISCHARGE END OF THE PUMP HOSE. THE TOLERANCE ON ENTRAPPED AIR SHALL BE +2.0% AND -1.5% WITH THE AVERAGE OF ALL TESTS NOT LESS THAN THE SPECIFIED AMOUNT.

TOTAL CEMENTITIOUS MATERIAL: THE SUM OF ALL CEMENT PLUS FLYASH AND SLAG. AT THE CONTRACTORS OPTION FLYASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT BUT SHALL NOT EXCEED 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL. IN NO CASE SHALL THE AMOUNT OF FLYASH OR SLAG BE LESS THAN REQUIRED BY THE CONCRETE MIX DESIGN TABLE. FOOTING MIXES SHALL CONTAIN NOT LESS THAN **5 SACKS** OF CEMENTITIOUS MATERIAL PER CUBIC YARD, ALL OTHER MIXES SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENTITIOUS MATERIAL PER CUBIC YARD. UNLESS NOTED OTHERWISE.

| ITEM | DESIGN fc (PSI) (AT 28 DAYS U.N.O.) | MAX. W/C RATIO | MIN. FLYASH OR SLAG (PCY) | AGGREGATE GRADING ASTM AASHTO | NOTES |
|---------------------|--|----------------------|------------------------------------|-------------------------------------|-------|
| SLABS ON METAL DECK | 4000 | 0.45 | 100 | 57 OR 67 | |
| OTHER CONC. | 4000 | 0.50 | - | 57 OR 67 | |

CONCRETE PLACEMENT

PLACE CONCRETE FOLLOWING ALL APPLICABLE ACI RECOMMENDATIONS. CONCRETE SHALL BE PROPERLY CONSOLIDATED PER ACI 309 USING INTERIOR MECHANICAL VIBRATORS, DO NOT OVER-VIBRATE. CONCRETE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION OR EXPANSION JOINTS. IF CONCRETE IS PLACED BY THE PUMP METHOD, HORSES SHALL BE PROVIDED TO SUPPORT THE HOSE, THE HOSE SHALL NOT BE ALLOWED TO RIDE ON THE REINFORCING. WEATHER FORECASTS SHALL BE MONITORED AND ACI RECOMMENDATIONS FOR HOT AND COLD WEATHER CONCRETING SHALL BE FOLLOWED AS REQUIRED. CONCRETE SHALL NOT FREE FALL MORE THAN 5 FEET DURING PLACEMENT WITHOUT WRITTEN APPROVAL OF STRUCTURAL ENGINEER.

COLD WEATHER PLACEMENT

- 1. COLD WEATHER IS DEFINED BY ACI 306 AS "A PERIOD WHEN FOR MORE THAN 3 SUCCESSIVE DAYS THE MEAN DAILY TEMPERATURE DROPS BELOW 40° F."
- NO CONCRETE SHALL BE PLACED ON FROZEN OR PARTIALLY FROZEN GROUND. THAWING THE GROUND
- 3. CONCRETE MIX TEMPERATURES SHALL BE AS SHOWN BELOW. HEATING OF WATER AND/OR AGGREGATES MAY BE REQUIRED TO ATTAIN THESE TEMPERATURES.
- 4. THE CONCRETE MAY REQUIRE PROTECTION FOR 4-7 DAYS AFTER POURING. IF TEMPERATURES REMAIN BELOW FREEZING, INSULATING BLANKET COVERAGE IS REQUIRED. IF TEMPERATURES ARE SLIGHTLY BELOW FREEZING (30° F MIN.) AT NIGHT AND ABOVE FREEZING DURING THE DAY, KRAFT PAPER WITH COMPLETE COVERAGE MAY BE USED IN LIEU OF INSULATED BLANKETS.
- NO ADDITIVES CONTAINING CHLORIDES SHALL BE USED. USE "POZZUTEC 20+" BY MASTER BUILDERS OR "POLARSET" BY W.R. GRACE OR PRE-APPROVED EQUAL.

| CONDITION OF PLACEMENT AND CURING | | WALLS & SLABS | FOOTINGS |
|---|---|-------------------|-------------------|
| MIN. TEMP. FRESH CONCRETE AS MIXED FOR WEATHER INDICATED, DEGREES F. | ABOVE 30° F. 0° TO 30° F. BELOW 0° F. | 60° 65° 70° | 55° 60° 65° |
| MIN. TEMP. FRESH CONCRETE AS PLACED AN | D MAINTAINED, DEGREES F. | 55° | 50° |
| MAX. ALLOWABLE GRADUAL DROP IN TEMP. T HOURS AFTER END OF PROTECTION, DEGREE | | 50° | 40° |

HOT OR WINDY WEATHER PLACEMENT

HOT WEATHER IS DEFINED BY ACI 305 AS "ANY COMBINATION OF HIGH AIR TEMPERATURE, LOW RELATIVE HUMIDITY, AND WIND VELOCITY, TENDING TO IMPAIR THE QUALITY OF FRESH HARDENED CONCRETE. ACI 305 FIGURE 2.1.5 SHALL BE USED BY THE CONTRACTOR TO ESTIMATE THE RATE OF EVAPORATION. WHEN THE ESTIMATED RATE OF EVAPORATION IS GREATER THAN 0.2 PSF/HOUR THE PLACEMENT SHALL BE CONSIDERED A HOT WEATHER PLACEMENT. PRECAUTIONS AGAINST PLASTIC SHRINKAGE CRACKING ARE NECESSARY. PRECAUTIONS TAKEN BY THE CONTRACTOR VARY DEPENDING UPON THE FACTORS ASSOCIATED WITH WATER **EVAPORATION AND INCLUDE BUT ARE NOT LIMITED TO:**

- LIMITING CONCRETE TEMPERATURE TO 100°F AT TIME OF PLACEMENT.
- 2. APPLICATION OF AN EVAPORATION RETARDER.
- 3. USE OF FOG SPRAY.
- 4. REDUCTION OF POUR SIZE
- 5. PLACING CONCRETE AT NIGHT

CONCRETE CURING AND SEALING

CURING PROCEDURES SHALL COMMENCE IMMEDIATELY AFTER FINISHING CONCRETE TO MAINTAIN CONCRETE IN A MOIST CONDITION. VERIFY CURING AND/OR SEALING PRODUCTS ARE COMPATIBLE WITH FLOOR COVERINGS SHOWN ON THE ARCHITECTURAL DRAWINGS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. SLABS ARE DEFINED AS SLABS ON GRADE, CONCRETE ON METAL DECK, ELEVATED POST-TENSIONED OR MILD REINFORCED DECKS, AND TOPPING SLABS.

| ITEM | CONCRETE CURING NOTES |
|--------------------------|-----------------------|
| SLABS EXPOSED TO WEATHER | 1, (3 OR 4 OR 5), 6 |

CONCRETE CURING NOTES:

- WHEN THE ESTIMATED EVAPORATION RATE IS GREATER THAN 0.2 PSF/HOUR PROVIDE A SPRAY APPLIED EVAPORATION RETARDER IMMEDIATELY AFTER CONCRETE PLACEMENT. THE EVAPORATION RATE MAY BE CALCULATED PER ACI 305 FIGURE 2.1.5.
- APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS TO ALL FORMED SURFACES IMMEDIATELY AFTER FINAL FORM REMOVAL. NOT REQUIRED IF FORMWORK REMAINS IN PLACE FOR MORE THAN 7 DAYS.
- PROVIDE PRE-APPROVED CONTINUOUS WET CURE METHOD FOR A MINIMUM OF 14 DAYS.
- APPLY A LIQUID MEMBRANE FORMING CURING COMPOUND, CONFORMING TO ASTM C309 TYPE 1 CLASS B SPECIFICATIONS OR ASTM C1315 TYPE 1 CLASS A SPECIFICATIONS, PER MANUFACTURER'S RECOMMENDATIONS IMMEDIATELY AFTER FINAL FINISHING. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR COVERINGS AND SEALERS.
- PROVIDE 'ULTRACURE MAX' MOISTURE RETAINING COVER BY MCTECH GROUP, OR APPROVED EQUAL, FOR A MINIMUM OF 14 DAYS.
- APPLY A SILANE SEALER WITH MINIMUM SOLIDS CONTENT OF 40% PER MANUFACTURER'S RECOMMENDATIONS.

<u>GROUT</u>

NON-SHRINK GROUT: MASTER BUILDERS "MASTERFLOW 928" OR PRE-APPROVED EQUAL. GROUT SHALL CONFORM TO CRD-C621 AND ASTM C1107 WHEN TESTED AT A FLUID CONSISTENCY PER CRD-C611-85 FOR 30 MINUTES. GROUT MAY BE PLACED FROM A 25 SECOND FLOW TO A STIFF PACKING CONSISTENCY. FILL OR PACK ENTIRE SPACE UNDER PLATES OR SHAPES. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR PREPARATION, INSTALLATION, AND CURING.

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO:

ASTM A615, GRADE 60 TYPICAL UNLESS NOTED OTHERWISE.

DETAIL FABRICATE AND PLACE PER ACI 315 AND ACI 318.

WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185. LAP ONE FULL MESH ON SIDES AND ENDS BUT NOT LESS THAN 8 INCHES. WELDED WIRE REINFORCING SHALL BE SUPPORTED TO WITHSTAND CONCRETE PLACEMENT. PULLING OF MESH INTO PLACE AFTER PLACEMENT IS NOT ALLOWED.

| <u>R</u> | EINFORCING SPLICE | AND DEVELOPMENT LE | ENGTH SCHEDULE, Fy= | 60 KSI (UNLESS NOTED | OTHERWISE) |
|----------|-------------------|--------------------|---------------------|----------------------|---|
| BAR | MINIMUM LAP SPLI | ICE LENGTHS ("Ls") | MINIMUM DEVELOP | MENT LENGTHS ("Ld") | MINIMUM EMBEDMENT |
| SIZE | TOP BARS (1) | OTHER BARS | TOP BARS (1) | OTHER BARS | - LENGTH FOR STANDARD END HOOKS ("Ldh") |
| #3 | 2'-0" | 1'-6" | 1'-6" | 1'-3" | 0'-7" |
| #4 | 2'-8" | 2'-0" | 2'-0" | 1'-7" | 0'-9" |
| #5 | 3'-4" | 2'-7" | 2'-7" | 2'-0" | 1'-0" |
| #6 | 4'-0" | 3'-1" | 3'-1" | 2'-4" | 1'-2" |
| #7 | 5'-10" | 4'-6" | 4'-6" | 3'-6" | 1'-5" |
| #8 | 6'-8" | 5'-2" | 5'-2" | 3'-11" | 1'-7" |
| #9 | 7'-6" | 5'-10" | 5'-10" | 4'-6" | 1'-9" |
| #10 | 8'-6" | 6'-6" | 6'-6" | 5'-0" | 2'-0" |
| #11 | 9'-5" | 7'-3" | 7'-3" | 5'-7" | 2'-3" |

| STRUCTURAL DRAWING INDEX | |
|--------------------------|-------------------|
| SHEET NUMBER | SHEET DESCRIPTION |
| S1.0 | GENERAL NOTES |
| S1.1 | GENERAL NOTES |
| S1.2 | GENERAL NOTES |
| S2.0 | PLANS |
| S4.0 | DETAILS |
| S4.1 | DETAILS |
| S4.2 | DETAILS |
| Grand total: 7 | |

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PC-1S **ELEVATOR AND STAIR** REPLACEMENT PIKE PLACE MARKET PDA



85 PIKE STREET, #500

SEATTLE, WA 98101

PERMIT SET M

| Date | Issue / Revision |
|--------|------------------|
| 6/3/21 | PERMIT SET |
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| CONT | ENTS: |
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NOTES

SCALE: 12" = 1'-0" PROJECT NO.: 21-102 FILE: SHEET:

SPLICE TABLE NOTES

1. "TOP BARS" ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM.

CONCRETE INSERTS: THREADED DOWEL BAR SUBSTITUTIONS SHALL BE MANUFACTURED BY RICHMOND SCREW ANCHOR CO., INC., OR PRE-APPROVED EQUAL AND SHALL BE CAPABLE OF DEVELOPING THE FULL TENSILE CAPACITY OF THE BAR.

POST-INSTALLED ANCHORS

POST-INSTALLED ANCHORS: SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH REBAR. INSTALL IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS. INSTALLER SHALL BE QUALIFIED AND TRAINED BY THE MANUFACTURER. HOLES SHALL BE HAMMER DRILLED ONLY (ROTARY DRILLED ONLY AT UNREINFORCED MASONRY - NO HAMMER TOOLS).

SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED FOR APPROVAL A MINIMUM OF 2 WEEKS PRIOR TO BID, ALONG WITH CALCULATIONS SHALL BE STAMPED BY A PROFESSIONAL ENGINEER (LICENSED IN THE STATE OF THE PROJECT) DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

CONCRETE ANCHORS

- ADHESIVE ANCHORS: HILTI HIT-HY 200 (ICC-ESR-3187), HILTI HIT-RE 500 V3 (ICC-ESR-3814), DEWALT PURE
 - 110+ (ICC-ESR-3298), OR SIMPSON SET-3G (ICC-ESR-4057), OR PRE-APPROVED EQUAL. *CONCRETE SHALL BE A MINIMUM OF 21 DAYS OLD AT TIME OF INSTALLATION.
 - *CONCRETE SHALL BE IN THE TEMPERATURE RANGE AS REQUIRED BY THE CONCRETE MANUFACTURER.
 - *HOLE SHALL BY HAMMER-DRILLED ONLY.
 - *DO NOT INSTALL IN WATER-FILLED HOLES.

INSTALLER CERTIFICATION PROGRAM.

- *INSTALLER OF HORIZONTAL OR UPWARDLY INCLINED (ANY POSITION EXCEPT DIRECTLY DOWNWARD) ANCHORS SHALL ALSO BE CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR
- EXPANSION ANCHORS: KWIKBOLT TZ (ICC ESR-1917) BY HILTI, INC. OR PRE-APPROVED EQUAL. SCREW ANCHORS: KWIK HUS-EZ (ICC ESR-3027) BY HILTI, INC. OR PRE-APPROVED EQUAL.

MASONRY ANCHORS (SOLID GROUTED MASONRY)

ADHESIVE ANCHORS: HILTI HIT-HY 270 (ICC-ESR-4143) OR PRE-APPROVED EQUAL.
 EXPANSION ANCHORS: KWIKBOLT III (ICC ESR-1385) BY HILTI, INC., OR PRE-APPROVED EQUAL
 SCREW ANCHORS: KWIK HUS-EZ (ICC ESR-3056) BY HILTI, INC., OR PRE-APPROVED EQUAL.

MASONRY ANCHORS (HOLLOW MASONRY):

- ADHESIVE ANCHORS: HILTI HIT-HY 270 WITH SCREEN TUBES AT HOLLOW CMU & UNREINFORCED BRICK MASONRY (ICC-ESR-4143 & ICC-ESR-4144) BY HILTI, INC. OR PRE-APPROVED EQUAL USING THE APPROPRIATE SIZE SCREEN TUBE REQUIRED BY THE MANUFACTURER.

STRUCTURAL STEEL

DETAILING, FABRICATION AND ERECTION

ALL WORKMANSHIP SHALL CONFORM TO THE AISC MANUAL OF STEEL CONSTRUCTION, 15TH EDITION, THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS JULY 7, 2016, THE AISC CODE OF STANDARD PRACTICE, JUNE 15, 2016 AND THE AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, JULY 12, 2016.

STEEL MEMBERS ARE EQUALLY SPACED BETWEEN COLUMNS AND/OR DIMENSION POINTS UNLESS NOTED OTHERWISE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDES AND JOINT PREPARATIONS THAT INCLUDE BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDES, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, WELD EXTENSION TABS, COPES, SURFACE ROUGHNESS VALUES AND TAPERS OF UNEQUAL PARTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLIANCE WITH ALL CURRENT OSHA REQUIREMENTS.

HOLES, COPES OR OTHER CUTS OR MODIFICATIONS OF THE STRUCTURAL STEEL MEMBERS SHALL NOT BE MADE IN THE FIELD WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.

STEEL FABRICATORS

NON-AISC CERTIFIED STEEL FABRICATORS SHALL HAVE FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

STEEL ERECTORS

NON-AISC CERTIFIED STEEL ERECTORS SHALL HAVE FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

STEEL DETAILERS

ALL STEEL DETAILING SHALL BE PERFORMED BY A DETAILER WITH FIVE YEARS MINIMUM EXPERIENCE ON SIMILAR PROJECTS OF EQUAL OR LARGER COMPLEXITY AND SCOPE. QUALIFICATIONS SHALL BE SUBMITTED TWO WEEKS PRIOR TO BID.

MATERIAL PROPERTIES

WIDE FLANGE SECTIONS: ASTM A992 (Fy = 50 KSI)

OTHER SHAPES AND PLATES: ASTM A36 (Fy = 36 KSI) TYP. U.N.O.; ASTM A572 (Fy = 50 KSI) WHERE INDICATED

HOLLOW STRUCTURAL SECTIONS: RECTANGULAR & SQUARE - ASTM A500 GRADE C (Fy = 50 KSI) ROUND - ASTM A500 GRADE C (Fy = 46 KSI)

STRUCTURAL STEEL PIPES: ASTM A53, GRADE B, TYPE E OR S (Fy = 35 KSI)

MACHINE BOLTS (M.B.): ASTM A307, GRADE A

<u>HIGH-STRENGTH BOLTS</u>: ASTM F3125, GRADE F1852, UNLESS NOTED OTHERWISE, ASTM F3125, GRADE F2280 WHERE INDICATED

ANCHOR BOLTS (A.B.): ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE, ASTM F1554, GRADE 105 WHERE INDICATED.

WELDING

STRUCTURAL STEEL: WELD IN ACCORDANCE WITH "STRUCTURAL WELDING CODE" AWS D1.1.

<u>LATERAL FORCE-RESISTING SYSTEM</u>: WELD IN ACCORDANCE WITH "STRUCTURAL WELDING CODE SEISMIC SUPPLEMENT" AWS D1.8.

<u>CERTIFICATION</u>: ALL WELDING SHALL BE PERFORMED BY WABO/AWS CERTIFIED WELDERS. WELDERS SHALL BE PREQUALIFIED FOR EACH POSITION AND WELD TYPE WHICH THE WELDER WILL BE PERFORMING.

WELD TABS (ALSO KNOWN AS WELD "EXTENSION" TABS OR "RUN OFF" TABS) SHALL BE USED. AFTER THE WELD HAS BEEN COMPLETED THE WELD TABS SHALL BE REMOVED AND THE WELD END GROUND TO A SMOOTH CONTOUR. WELD "DAMS" OR "END DAMS" SHALL NOT BE USED.

THE PROCESS CONSUMABLES FOR ALL WELD FILLER METAL INCLUDING TACK WELDS, ROOT PASS AND SUBSEQUENT PASSES DEPOSITED IN A JOINT SHALL BE COMPATIBLE.

ALL WELD FILLER METAL AND WELD PROCESS SHALL PROVIDE THE TENSILE STRENGTH AND CHARPY V-NOTCH RATINGS AS FOLLOWS:

GRAVITY FRAME

| WELD TYPE | FILLER METAL TENSILE STRENGTH | CHARPY V-NOTCH (CVN) RATING |
|----------------------|-------------------------------|-----------------------------|
| FILLET | 70 KSI | |
| PARTIAL PENETRATION | 70 KSI | |
| COMPLETE PENETRATION | 70 KSI | 20 FT-LBS @ 40 DEG F |

WELDED CONNECTIONS INSPECTION:

- 1. ALL WELDING SHALL BE CHECKED BY VISUAL MEANS AND BY OTHER METHODS DEEMED NECESSARY BY THE WELDING INSPECTOR.
- 2. ALL FULL PENETRATION WELDS TO MEMBERS WHICH FORM A PORTION OF THE LATERAL FORCE-RESISTING SYSTEM SHALL BE CHECKED 100 PERCENT BY ULTRASONIC TESTING.
- THE CONTRACTOR SHALL SUBMIT A WRITTEN WELDING PROCEDURE SPECIFICATION FOR SHOP AND FIELD WELDING OF ALL LATERAL FORCE-RESISTING SYSTEM CONNECTIONS FOR APPROVAL TO THE STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION.

THE STANDARDS OF ACCEPTANCE FOR WELDS TESTED BY ULTRASONIC METHODS SHALL CONFORM TO AWS D1.1.

ALL WELDS FOUND TO BE DEFECTIVE SHALL BE REPAIRED AND REINSPECTED BY THE SAME METHODS ORIGINALLY USED, AND THIS REPAIR AND REINSPECTION SHALL BE PAID FOR BY THE CONTRACTOR

GENERAL REQUIREMENTS

HIGH-STRENGTH BOLTS: ALL A325 HIGH-STRENGTH BOLTS (HSB) SHALL BE ASTM F3125, GRADE F1852, UNLESS OTHERWISE DESIGNATED AS A490. ALL HSB DESIGNATED AS A490 SHALL BE ASTM F3125, GRADE F2280. ALL HSB SHALL BE BY "LEJEUNE BOLT COMPANY" OR PRE-APPROVED EQUAL AND SHALL BE INSTALLED PER SECTION 8.2 OF THE "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS", AUGUST 2014 BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC SPECIFICATION). ALL BOLT HOLES SHALL BE STANDARD ROUND HOLES UNLESS NOTED OTHERWISE. THE FAYING SURFACES OF ALL PLIES WITHIN THE GRIP OF SLIP-CRITICAL BOLTS (A325SC OR A490SC) SHALL MEET THE REQUIREMENTS FOR A CLASS A SURFACE PER SECTION 3.2 OF THE RCSC SPECIFICATION.

<u>BOLTED CONNECTIONS INSPECTION</u>: CONNECTIONS MADE WITH BEARING TYPE BOLTS SHALL BE INSPECTED PER SECTION 9.1 AND CONNECTIONS MADE WITH SLIP-CRITICAL TYPE BOLTS (A325SC OR A490SC) SHALL BE INSPECTED PER SECTION 9.3 OF RCSC SPECIFICATION.

ADHESIVE ANCHOR RODS: ASTM F1554, GRADE 36 UNLESS NOTED OTHERWISE.

HEADED STUDS: SHALL BE "H4L HEADED CONCRETE ANCHORS" FOR STUDS 5/8" DIAMETER AND SMALLER AND "S3L SHEAR CONNECTORS" FOR STUDS 3/4" DIAMETER AND LARGER AS MANUFACTURED BY NELSON STUD WELDING, INC. OR PRE-APPROVED EQUAL AND SHALL CONFORM TO AWS D1.1. ALL HEADED STUDS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS USING A NELSON WELD GUN, UNLESS NOTED OTHERWISE ON DETAILS. ALL WELDS SHALL BE MADE AND INSPECTED IN ACCORDANCE WITH AWS D1.1.

<u>DEFORMED BAR ANCHORS</u>: SHALL BE "D2L DEFORMED BAR ANCHORS" AS MANUFACTURED BY NELSON STUD WELDING, INC. OR PRE-APPROVED EQUAL AND SHALL CONFORM TO AWS D1.1. ALL DEFORMED BAR ANCHORS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS USING A NELSON WELD GUN, UNLESS NOTED OTHERWISE ON DETAILS. ALL WELDS SHALL BE MADE AND INSPECTED IN ACCORDANCE WITH AWS D1.1.

FINISH: STRUCTURAL STEEL SHALL BE UNPAINTED, UNLESS NOTED OTHERWISE, AND SHALL BE CLEAN OF LOOSE RUST, LOOSE MILL SCALE, OIL, GREASE AND OTHER FOREIGN SUBSTANCES AND SHALL MEET THE REQUIREMENTS OF SSPC-SP1. WHERE STRUCTURAL STEEL IS NOTED TO BE PAINTED, ALL AREAS COMPRISING THE FAYING SURFACES OF BOLTED CONNECTIONS MADE WITH SLIP-CRITICAL TYPE BOLTS (A325SC OR A490SC) SHALL COMPLY WITH THE REQUIREMENTS OF THE RCSC SPECIFICATION. WHERE STRUCTURAL STEEL IS NOTED TO BE GALVANIZED, IT SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123, A384, AND A385. ALL SURFACES WITHIN TWO INCHES OF ANY FIELD WELD LOCATION SHALL BE FREE OF MATERIALS THAT WOULD PREVENT PROPER WELDING OR PRODUCE OBJECTIONABLE FUMES. FIELD TOUCH-UP OF PRIMED, PAINTED, AND GALVANIZED SURFACES SHALL BE PERFORMED TO REPAIR COATING ABRASIONS, AS WELL AS TO PROTECT ALL AREAS AT CONNECTIONS.

ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS): ALL MEMBERS DESIGNATED AS AESS SHALL CONFORM TO SECTION 10, ARCHITECTURALLY EXPOSED STRUCTURAL STEEL, OF THE AISC CODE OF STANDARD PRACTICE.

COMPOSITE FLOOR DECK: SHALL CONTAIN THE MINIMUM PROPERTIES SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE "FORMLOK" AS MANUFACTURED BY VERCO MANUFACTURING CO., "W COMPOSITE" AS MANUFACTURED BY ASC STEEL DECK, "EPICORE" AS MANUFACTURED BY EPIC METALS, OR PRE-APPROVED EQUAL. THE FLOOR UNITS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A653, AND GALVANIZED PER ASTM A924. SUBMIT SHOP DRAWINGS SHOWING LAYOUT AND FASTENING PATTERN. ALL ACCESSORIES INCLUDING EDGE FORMS, CLOSURE, ETC. SHALL BE PROVIDED TO COMPLETE THE INSTALLATION OF THE COMPOSITE FLOOR.

METAL ROOF DECK: SHALL CONTAIN THE MINIMUM PROPERTIES SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE MANUFACTURED BY VERCO MANUFACTURING CO., ASC STEEL DECK, EPIC METALS, OR PRE-APPROVED EQUAL. THE ROOF DECK SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A611 OR A653, AND SHALL BE GALVANIZED PER ASTM A924. THE ROOF DECK SHALL BE PLACED ON THE SUPPORTING FRAMEWORK WITH A MINIMUM END LAP OF TWO INCHES. SUBMIT SHOP DRAWINGS SHOWING LAYOUT AND FASTENING PATTERN. ALL ACCESSORIES SHALL BE PROVIDED TO COMPLETE THE ERECTION OF THE STEEL DECK.

PREFABRICATED METAL STAIRS AND LANDINGS: SHALL BE DESIGNED PER AISC AND IBC REQUIREMENTS. STRINGERS AND LANDINGS SHALL BE DESIGNED FOR A MINIMUM OF 100 PSF LIVE LOAD. INDIVIDUAL TREADS SHALL BE DESIGNED FOR 300 POUND CONCENTRATED LOAD. EGRESS STAIRS AND LANDINGS SHALL BE DESIGNED WITH CONNECTIONS THAT ALLOW FOR SLIDING OR PROVIDE DUCTILITY TO ACCOMMODATE A HORIZONTAL SEISMIC DRIFT OF BETWEEN EACH LEVEL PER SECTION 13.5.10 OF ASCE 7-16. THE STAIR DESIGN CALCULATIONS AND SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD SHOWING THE COMPLETE ASSEMBLY AND ATTACHMENTS TO THE SURROUNDING STRUCTURE. THE ATTACHMENTS SHALL BE DETAILED SUCH THAT NO TORQUE IS APPLIED TO THE SURROUNDING STRUCTURAL MEMBERS. THE CALCULATIONS AND SHOP DRAWINGS SHALL BE STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF THE PROJECT.

MISCELLANEOUS:

PRE-APPROVED SUBSTITUTIONS: SUBSTITUTIONS MAY BE ALLOWED ONLY IF THEY MEET THE REQUIREMENTS OF THESE GENERAL NOTES AND THE SPECIFICATIONS, AND IF COMPLETE WRITTEN ENGINEERING DATA FOR EACH CONDITION REQUIRED FOR THIS PROJECT IS PROVIDED TO THE STRUCTURAL ENGINEER TWO WEEKS PRIOR TO BID DATE AND APPROVED IN WRITTEN ADDENDA BY THE ARCHITECT. DATA IS TO INDICATE CODE BASIS BY YEAR, AUTHORITY FOR STRESSES AND STRESS INCREASES, IF ANY, AND AMOUNT OF EXPECTED DEFLECTION FOR FLEXURAL MEMBERS UNDER (1) TOTAL LOAD AND (2) LIVE LOAD ONLY. ALL INCREASED COSTS IN MECHANICAL, SPRINKLER, ELECTRICAL OR GENERAL INSTALLATION AND ANY ARCHITECTURAL OR STRUCTURAL REDESIGN RESULTING FROM SUBSTITUTION SHALL BE BORNE BY THE GENERAL CONTRACTOR.

SHOP DRAWINGS/SUBMITTALS

THE FOLLOWING SHOP DRAWINGS/SUBMITTALS SHALL BE PROVIDED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION OR DELIVERY.

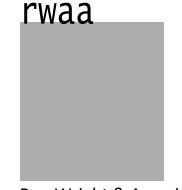
| | | STRUCTURAL ENGR. | BLDG. DEPT. |
|----|--|------------------|-------------|
| 1. | CONCRETE MIX DESIGNS | X | X |
| 2. | REINFORCING STEEL SHOP DRAWINGS | X | |
| 3. | STRUCTURAL STEEL | X | X |
| 4. | METAL DECK | X | X |
| 5. | PREFABRICATED METAL STAIRS AND LANDINGS | X | X |
| 6. | MISCELLANEOUS STEEL | X | X |
| 7. | CONTRACTOR'S STATEMENT OF RESPONSIBILITY | X | X |

DEFERRED SUBMITTALS

THE FOLLOWING ARE NOT INCLUDED WITH THE BUILDING PERMIT DRAWINGS AND SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL AS A DEFERRED SUBMITTAL SUBMITTALS SHALL BE STAMPED BY AN ENGINEER LICENSED IN THE STATE OF THE PROJECT AS NOTED.

| | | ENGINEER STAMP REQUIRED |
|----|---|-------------------------|
| 1. | PREFABRICATED METAL STAIRS AND LANDINGS | SE |

SPECIAL INSPECTION: SPECIAL INSPECTION SHALL BE PROVIDED BY AN INDEPENDENT TESTING LABORATORY PER THE REQUIREMENTS OF IBC CHAPTER 17 AND THE LOCAL BUILDING OFFICIAL OR APPLICABLE JURISDICTION AND THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SUBMIT INSPECTION REPORTS AND A FINAL SIGNED REPORT TO THE BUILDING OFFICIAL FOR THE ITEMS LISTED IN THE QUALITY ASSURANCE/SPECIAL INSPECTION SECTION:



Ron Wright & Associates / Architects, PS 2003 Western Ave., Suite 610 Seattle, Washington 98121-3133

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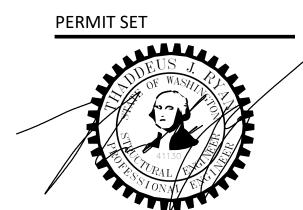
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AND STAIR
REPLACEMENT

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85 PIKE STREET, #500

SEATTLE, WA 98101





Date Issue / Revision
6/3/21 PERMIT SET

CONTENTS:
GENERAL

SCALE: 12" = 1'-0"

PROJECT NO.: 21-102

FILE:

SHEET:

NOTES

S1.1

TESTING OF MATERIALS

| STRUCTURAL SYSTEM | VERIFICATION AND INSPECTION | CONTINUOUS | PERIODIC | COMMENTS | REFERENCES |
|--|--|------------------|-------------|---|--|
| STEEL CONSTRUCTION | MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS | | Х | | AISC 360 CHAPTER N5 |
| | HIGH-STRENGTH BOLTING A. SNUG-TIGHT JOINTS B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST OFF BOLTS OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION | | X X | | AISC 360 CHAPTER N5 AISC 341 CHAPTER J7 |
| | MATERIAL VERIFICATION OF STRUCTURAL STEEL A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS | | X X | MANUFACTURER TO PROVIDE CERTIFIED MILL TEST REPORTS | AISC 360 CHAPTER N5 AISC 341 CHAPTER J6 |
| | MATERIAL VERIFICATION OF WELD FILLER MATERIALS A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATIONS LISTED IN GENERAL NOTES B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE | | X X | MANUFACTURER TO PROVIDE CERTIFICATE OF COMPLIANCE | AISC 360 CHAPTER N5 |
| | INSPECTION OF WELDING A. COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS B. MULTI-PASS FILLET WELDS C. SINGLE-PASS FILLET WELDS > 5/16" D. PLUG AND SLOT WELDS E. SINGLE-PASS FILLET WELDS ≤ 5/16" F. WELDING OF STAIRS AND RAILING SYSTEMS | X X X X | X X | SPECIAL INSPECTIONS IN THIS SECTION ARE WAIVED WHERE FABRICATION IS PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED IN ACCORDANCE WITH IBC SECTION 1704.2.5 | AISC 360 CHAPTER N5 AISC 341 CHAPTER J6 AWS D1.1 |
| STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL | MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK: A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS B. MANUFACTURER'S CERTIFIED TEST REPORTS | | X X | | APPLICABLE ASTM MATERIAL STANDARDS & IBC 2210.1.1 |
| 0.122 | INSPECTION OF WELDING A. COLD-FORM STEEL DECK WELDS B. REINFORCING STEEL: 1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706 2. REINFORCING STEEL IN MOMENT FRAMES AND BOUNDARY ELEMENTS 3. SHEAR REINFORCEMENT | X X | X X | | AWS D1.3 AWS D1.4 ACI 318:26.6.4 |
| | 4. OTHER REINFORCING STEEL 5. OPEN WEB STEEL JOISTS & JOIST GIRDERS A. END CONNECTIONS - WELDING OR BOLTED B. BRIDGING - HORIZONTAL OR DIAGONAL 1. STANDARD BRIDGING 2. BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1 | X | X X X | | IBC 1705.2.3 SJI SPECIFICATIONS LISTED IN SECTION 2207.1 SJI SPECIFICATIONS LISTED IN SECTION 2207.1 |
| CONCRETE | REINFORCING STEEL AND PLACEMENT | | Х | SPECIAL INSPECTIONS NOT REQUIRED FOR THE FOLLOWING CONDITIONS: | ACI 318: CH 20, 25.2, 25.3, 26.6-1 TO 26.6-3, IBC 1908.4 |
| | ANCHORS CAST IN CONCRETE-PRIOR TO AND DURING PLACEMENT OF CONCRETE | | Х | NON-STRUCTURAL SLAB ON GRADE | ACI 318: 17.8.2 AISC 360 SECTION N7 |
| | ANCHORS POST-INSTALLED IN HARDENED CONCRETE (MECHANICAL ANCHORS INSTALLED IN ANY DIRECTION AND ADHESIVE ANCHORS INSTALLED DOWNWARD) | | Х | PERIODIC INSPECTION TO INCLUDE A QUANTITY OF 10% WITH A MINIMUM OF (5) ANCHORS INSPECTED PER INSTALLER ON A DAILY BASIS. | ACI 318: 17.8.2 MFR EVAL REPORT MFR PUBLISHED INSTALLATION INSTRUCTIONS |
| | ANCHORS POST-INSTALLED IN HARDENED CONCRETE (ADHESIVE ANCHORS INSTALLED HORIZONTAL OR UPWARDLY INCLINED) | X | | | ACI 318: 17.8.2 MFR EVAL REPORT MFR PUBLISHED INSTALLATION INSTRUCTIONS |
| | VERIFY USE OF REQUIRED DESIGN MIX | | X | | ACI 318, CH 19 |
| | PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE | X | | | ASTM C172, C31 ACI 318: 26.4, 26.12 IBC 1908.10 |
| | CONCRETE PLACEMENT FOR PROPER APPLICATION | X | | | ACI 318: 26.5 IBC 1908.6, 1908.7, 1908.8 |
| | MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES | | Х | | ACI 318: 26.5.3 TO 26.5.5 IBC 1908.9 |
| | VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS | | X | | ACI 318: 26.11.2 |
| | TEOTING OF MATERIALS | | ., | | 150 /500 |

IBC 1705.3.2

TESTING AND SPECIAL INSPECTION REPORTS SHALL BE PREPARED FOR EACH INSPECTION ITEM ON A DAILY BASIS WHENEVER WORK IS PERFORMED ON THAT ITEM. REPORTS SHALL BE DISTRIBUTED TO OWNER, CONTRACTOR, BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD.

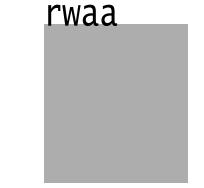
STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNATED REPRESENTATIVE IN ACCORDANCE WITH IBC 1704.6.

STRUCTURAL OBSERVATION SHALL BE PERFORMED AS FOLLOWS:

- » PERIODIC VISUAL OBSERVATION OF STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES.
- » REVIEW OF TESTING AND INSPECTION REPORTS.
- » REPORTS SHALL BE PREPARED FOR EACH SITE VISIT AND SHALL BE DISTRIBUTED TO ARCHITECT.

GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER PRIOR TO COMMENCEMENT OF WORK. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL INCLUDE ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTION.

| @ | AT | ELEV. | ELEVATOR | N.F. | NEAR FACE |
|------------------|----------------------------|--------|---------------------------|-------------|-------------------------------|
| A.B. | ANCHOR BOLT | ENGR. | ENGINEER | N.S. | NEAR SIDE |
| ADD'L | ADDITIONAL | EQ. | EQUAL | NTS | NOT TO SCALE |
| A.F.F. | ABOVE FINISH FLOOR | E.M. | EACH WAY | O.C. | ON CENTER |
| ALT. | ALTERNATE | EXP. | EXPANSION | OPN'G | OPENING |
| ARCH. | ARCHITECTURAL | EXT. | EXTERIOR | OPP. | OPPOSITE |
| BLD'G | BUILDING | FDN | FOUNDATION | P.A.F. | POWDER ACTUATED FASTENER |
| BLK'G | BLOCKING | F.F. | FAR FACE | PERP. | PERPENDICULAR |
| BM | BEAM | FLR | FLOOR | P | PLATE |
| B.O.F. | BOTTOM OF FOOTING | F.O.M. | FACE OF MASONRY | P.P. | PARTIAL PENETRATION |
| ВОТ. | ВОТТОМ | F.O.S. | FACE OF STUD | P.P.T. | PRESERVATIVE PRESSURE TREATED |
| BRG | BEARING | FRM'G | FRAMING | P.S.F. | POUNDS PER SQUARE FOOT |
| BTWN | BETWEEN | F.R.T. | FIRE RETARDANT TREATED | PSL | PARALLAM |
| B.V. | BUILT UP | F.S. | FAR SIDE | P.T. | POST TENSION |
| (C=) | CAMBER | FTG | FOOTING | PW. | PLYWOOD |
| CANT. | CANTILEVER | GA. | GAGE/GAUGE | REINF. | REINFORCING |
| C.F.S. | COLD-FORMED STEEL | GALV. | GALVANIZED | REQ'D | REQUIRED |
| C.J. | CONTROL/CONSTRUCTION JOINT | GL. | GLULAM | SCHED. | SCHEDULE |
| <u> </u> | CENTERLINE | GR. | GRADE | S.C.L.SHT'G | STRUCTURAL COMPOSITE LUMBI |
| CLR. | CLEARANCE | GWB | GYPSUM WALL BOARD | SHT'G | SHEATHING |
| CMU | CONCRETE MASONRY UNIT | HDR | HEADER | SIM. | SIMILAR |
| COL. | COLUMN | HGR | HANGER | S.O.G. | SLAB ON GRADE |
| CONC. | CONCRETE | HORIZ. | HORIZONTAL | SQ. | SQUARE |
| CONN. | CONNECTION | H55 | HOLLOW STRUCTURAL SECTION | STD | STANDARD |
| CONST. | CONSTRUCTION | HT | HEIGHT | STIFF. | STIFFENER |
| CONT. | CONTINUOUS | INT. | INTERIOR | STL | STEEL |
| CONTR. | CONTRACTOR | JST | JOIST | STRUCT. | STRUCTURAL |
| COORD. | COORDINATE | TL | JOINT | T&B | TOP & BOTTOM |
| C.P. | COMPLETE PENETRATION | L | ANGLE | T&G | TONGUE AND GROOVE |
| CTR'D | CENTERED | L.L. | LIVE LOAD | THR'D | THREADED |
| C.Y. | CUBIC YARD | LLH | LONG LEG HORIZONTAL | T.O.F. | TOP OF FOOTING |
| DBL. | DOUBLE | LLV | LONG LEG VERTICAL | T.O.S. | TOP OF STEEL |
| D.F. | DOUGLAS FIR | LOC. | LOCATION | TRT'D | TREATED |
| IA. <i>Ο</i> R Φ | DIAMETER | LSL | LAMINATED STRAND LUMBER | TYP. | TYPICAL |
| DIAG. | DIAGONAL | LVL | LAMINATED VENEER LUMBER | U.N.O. | UNLESS NOTED OTHERWISE |
| DIM. | DIMENSION | MAX. | MAXIMUM | U.T. | ULTRASONIC TESTED |
| D.L. | DEAD LOAD | M.B. | MACHINE BOLT | VERT. | VERTICAL |
| DWG | DRAWING | MECH. | MECHANICAL | M/ | MITH |
| DWL | DOWEL | MEZZ. | MEZZANINE | W.P. | WORK POINT |
| (E) | EXISTING | MFR | MANUFACTURER | MT | WEIGHT |
| EA. | EACH | MIN. | MINIMUM | W.W.R. | WELDED WIRE REINFORCING |
| E.F. | EACH FACE | MISC. | MISCELLANEOUS | | • |



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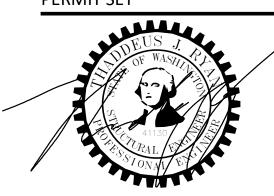
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PC-1S **ELEVATOR** AND STAIR REPLACEMENT

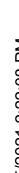


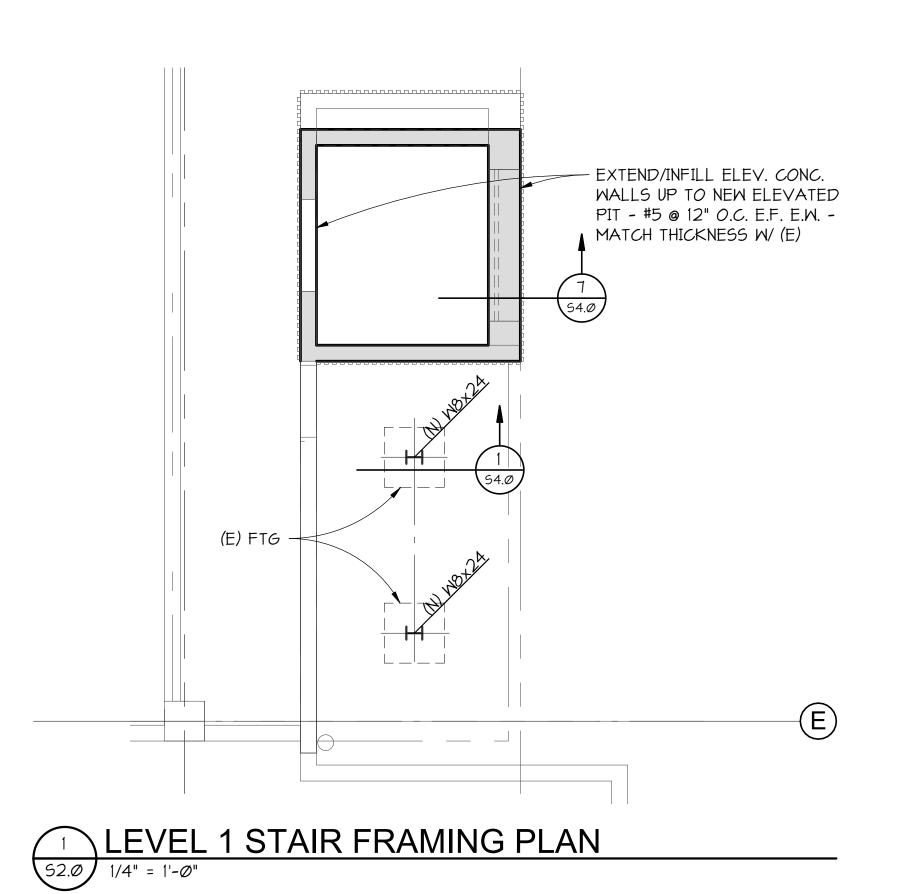
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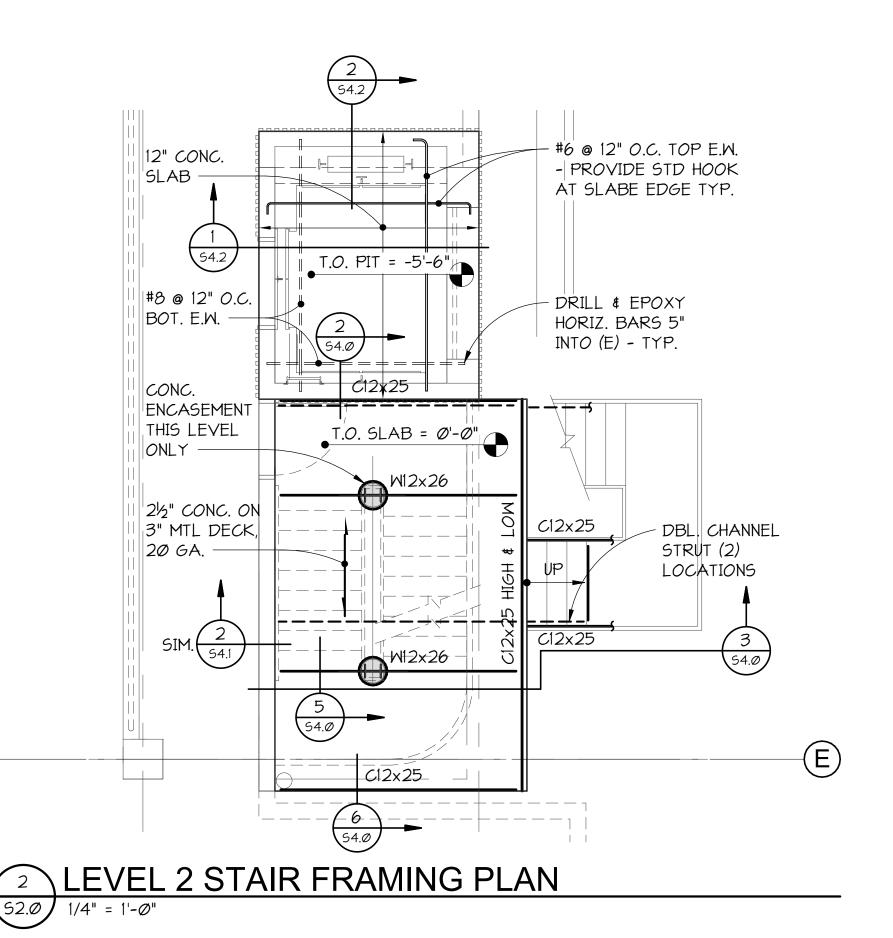


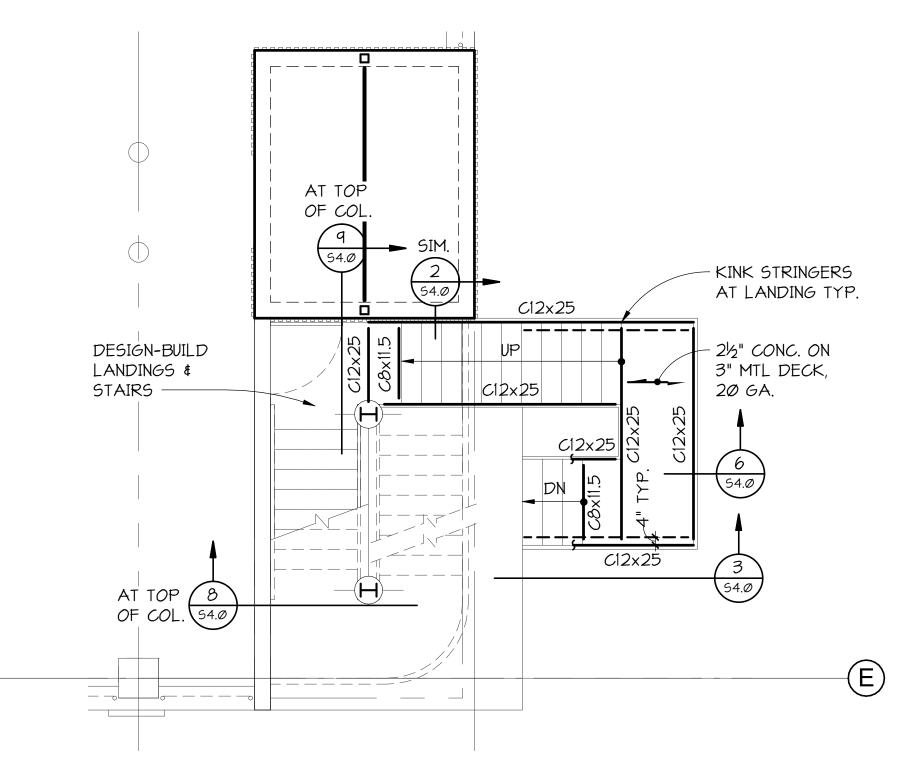
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3 LEVEL 3 STAIR FRAMING PLAN

52.0 1/4" = 1'-0"

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ELEVATOR
AND STAIR
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REPLACEMENT
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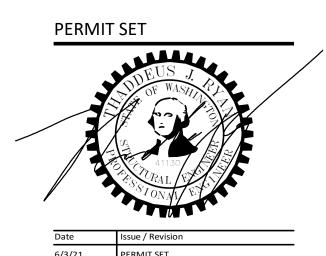
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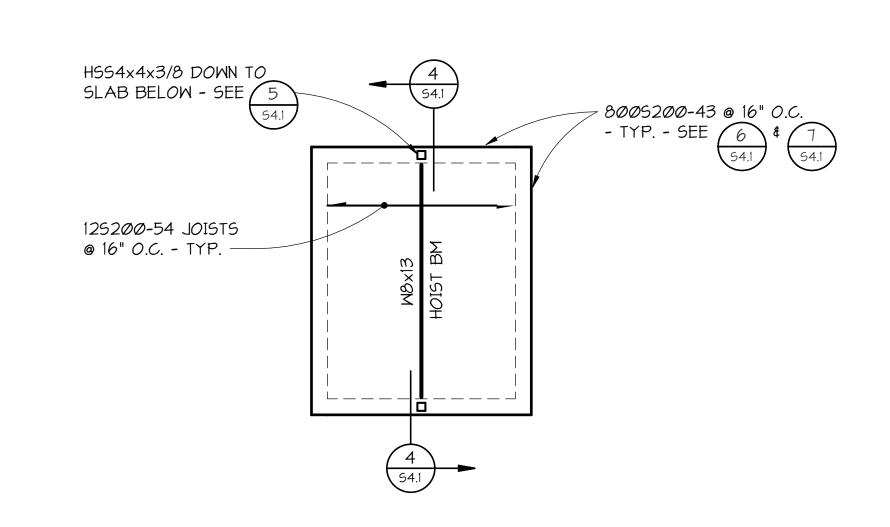
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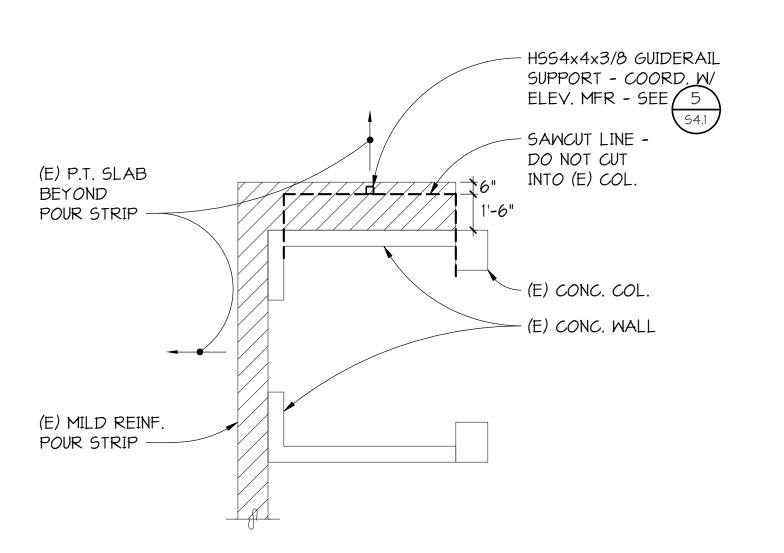
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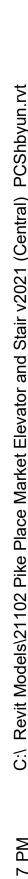
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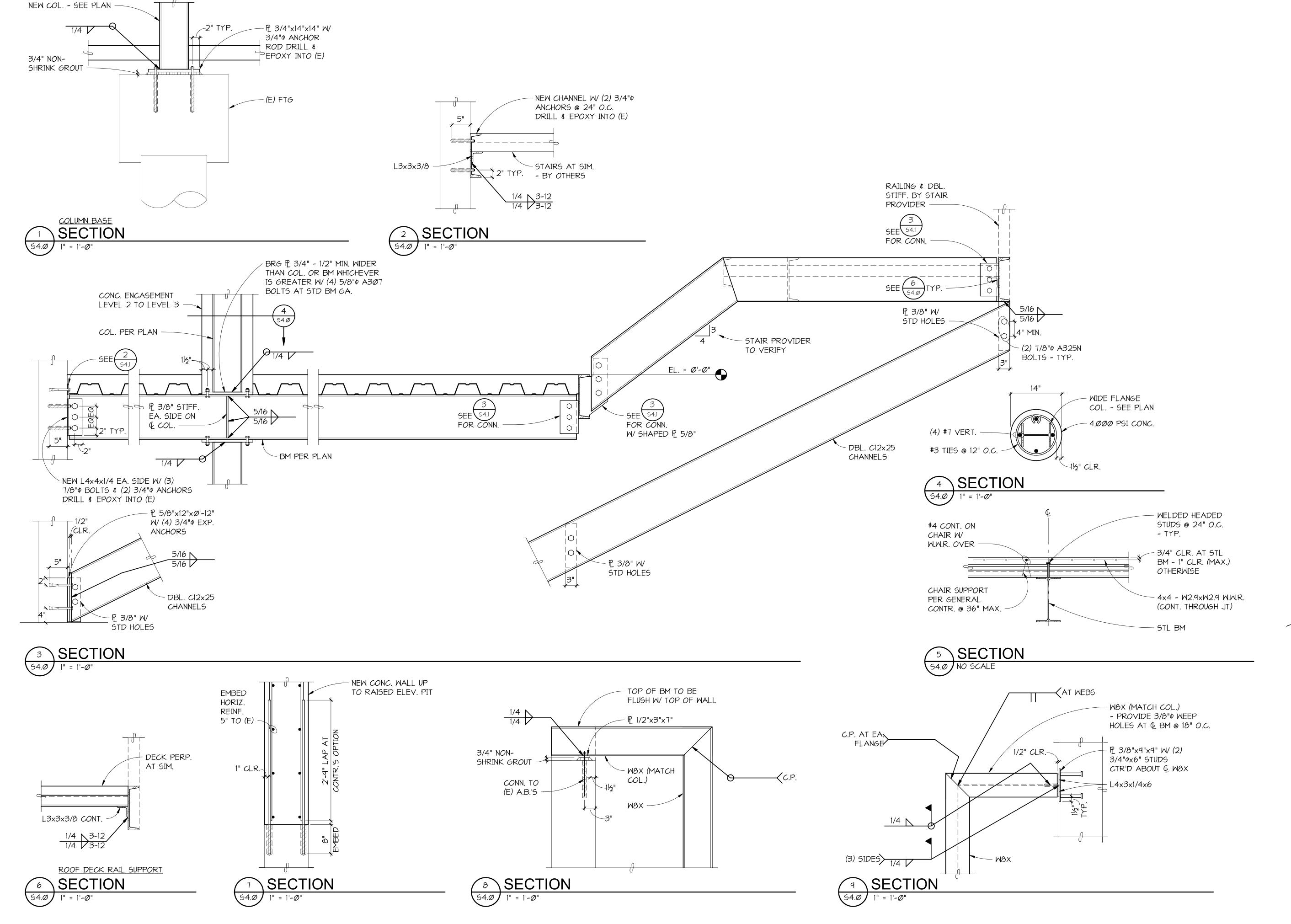












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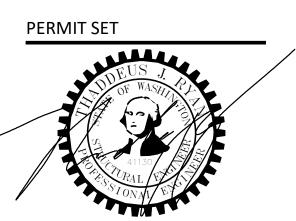
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PC-1S **ELEVATOR** AND STAIR REPLACEMENT



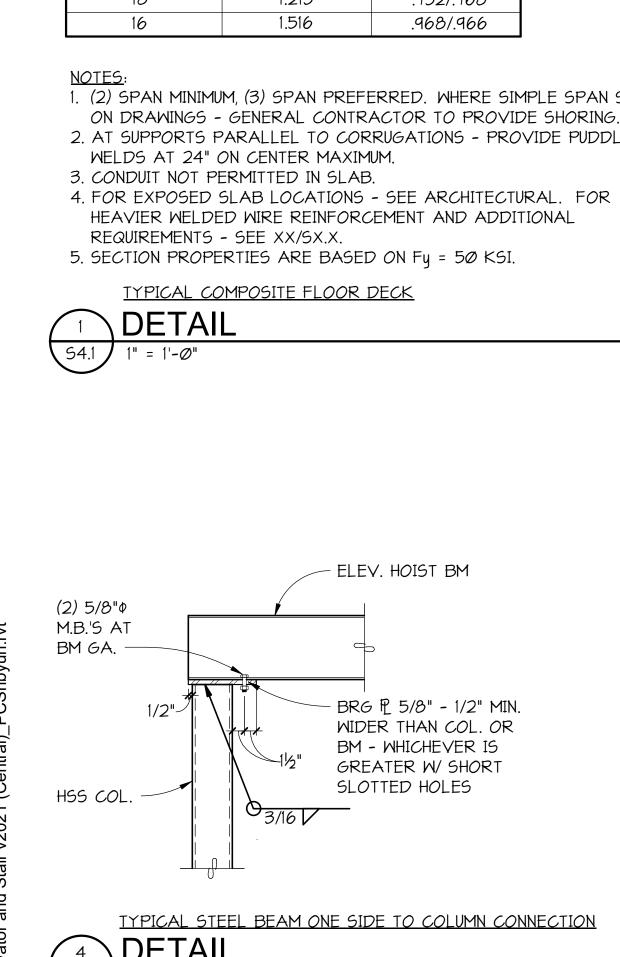




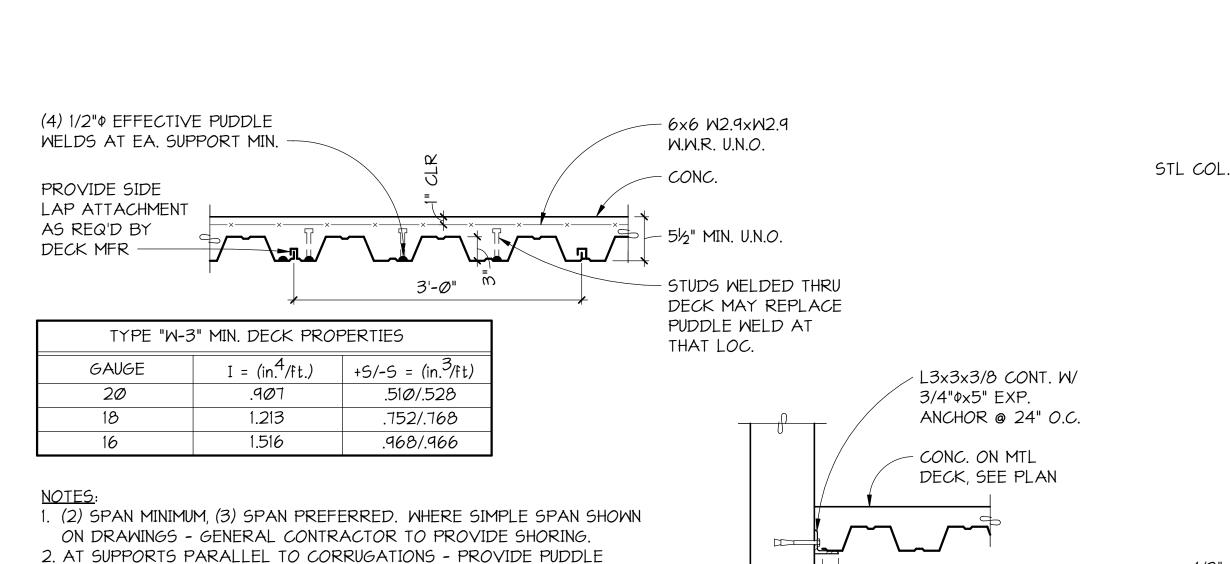
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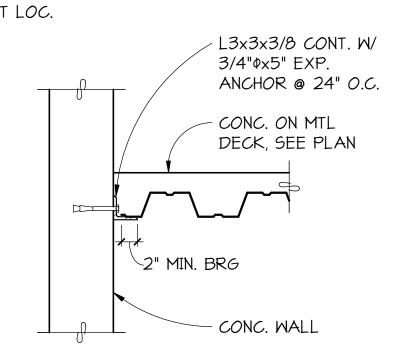
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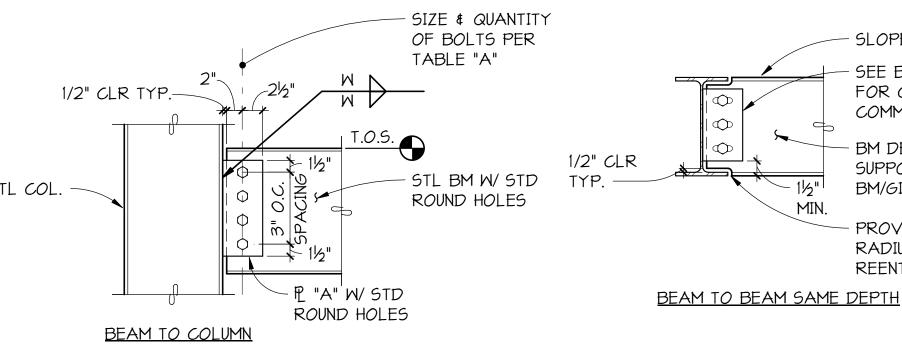
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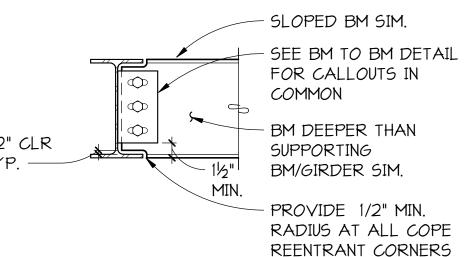












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SEE BM TO BM

DETAIL FOR

CALLOUTS IN

COMMON

| TABLE A | | | | |
|------------------------|--------------------------------------|---------------------------------|---------------------|--|
| SUPPORTED BEAM SIZE | QUANTITY OF 7/8"\$ A325N BOLTS | SHEAR PLATE THICKNESS "A" | WELD "W" NOTE 1. | |
| MIØ | 2 | 3/8" | 5/16" | |
| W12, W14 | 3 | 3/8" | 5/16" | |
| W16, W18 | 4 | 3/8" | 5/16" | |
| W21 | 5 | 3/8" | 5/16" | |
| W24 | 6 | 3/8" | 5/16" | |
| W27 | 7 | 3/8" | 5/16" | |
| W3Ø | 7 | 3/8" | 5/16" | |
| W33 | 8 | 3/8" | 5/16" | |

1. CONNECTIONS SIMILAR AT CHANNELS.

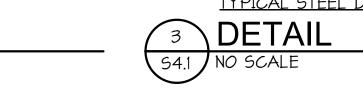
SIZE & QUANTITY OF BOLTS PER TABLE "A" 2" MIN. STL BM W/ STD ROUND HOLES - 3" O.C. 1/2" CLR TYP.-SPACING P "A" W/ HORIZ. SHORT SLOTTED BEAM TO BEAM HOLES

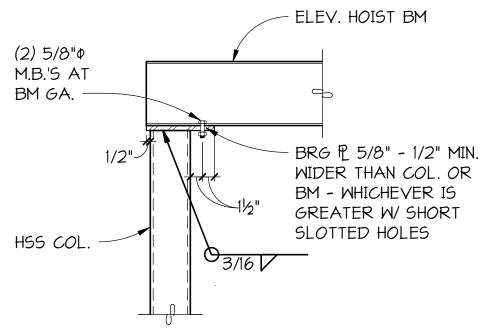
TYPICAL STEEL DETAIL 3 DETAIL

BM SIM. 1/2" MIN. RADIUS TYP. BEAM TO BEAM AT VARYING ELEVATION

SLOPED

SECTION





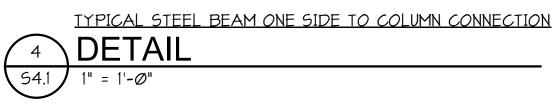
WELDS AT 24" ON CENTER MAXIMUM.

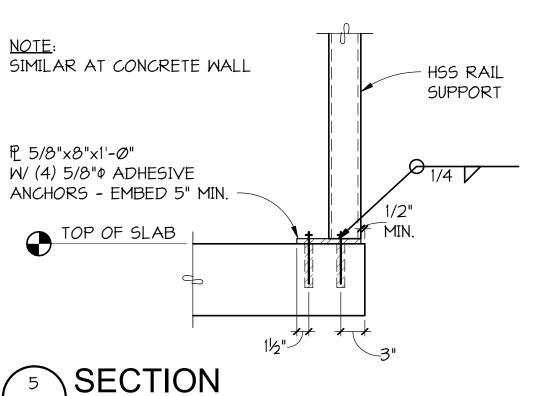
TYPICAL COMPOSITE FLOOR DECK

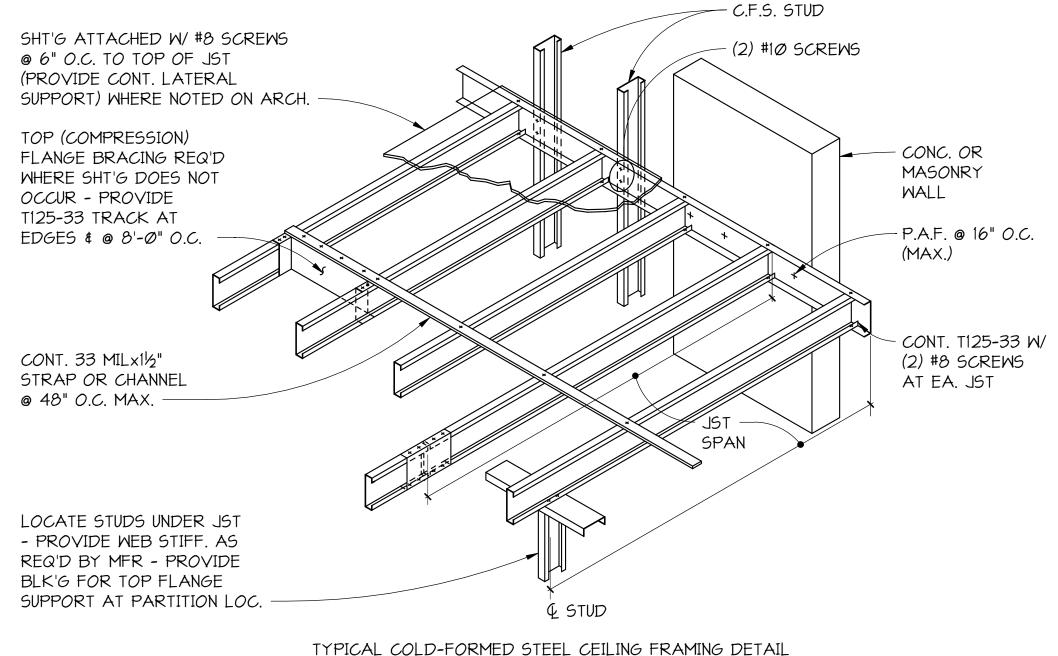
REQUIREMENTS - SEE XX/SX.X.

DETAIL

HEAVIER WELDED WIRE REINFORCEMENT AND ADDITIONAL

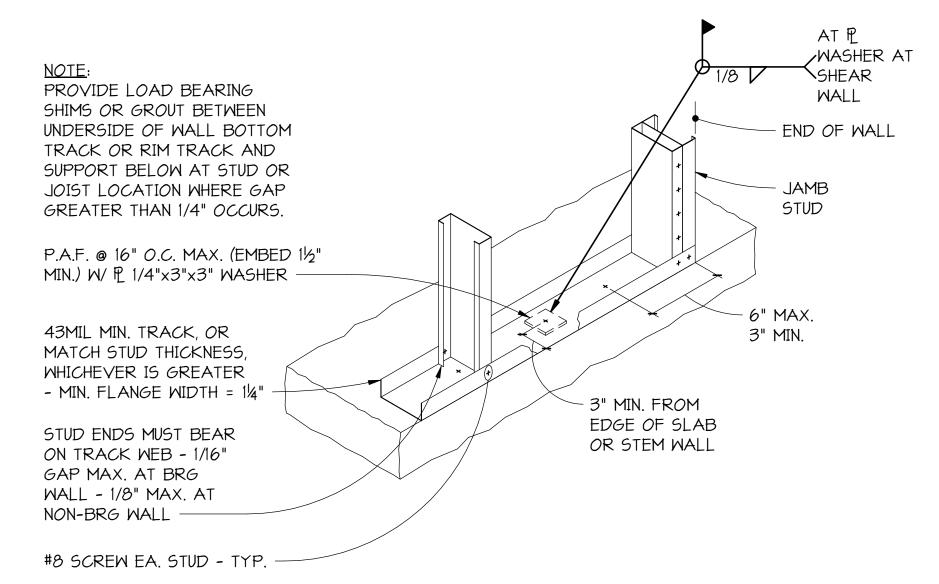






TYPICAL COLD-FORMED STEEL CEILING FRAMING DETAIL BRACE TOP FLANGE AT 48" ON CENTER MAXIMUM. 2. INFORMATION SHOWN TAKEN FROM THE GYPSUM CONSTRUCTION HAND BOOK BY CGC INCORPORATED. 3. SEE ARCHITECTURAL DRAWING FOR LATERAL SUPPORT OF SUSPENDED FRAMING.





TYPICAL NON-BEARING COLD-FORMED STEEL AND TRACK ATTACHMENT

DETAIL 54.1 NO SCALE

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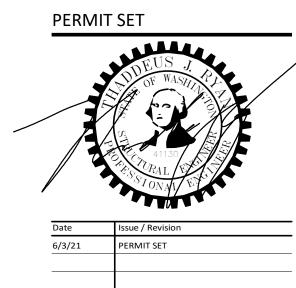
Architects, PS

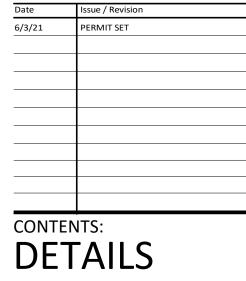
98121-3133

Seattle, Washington

PHONE (206) 728-4248



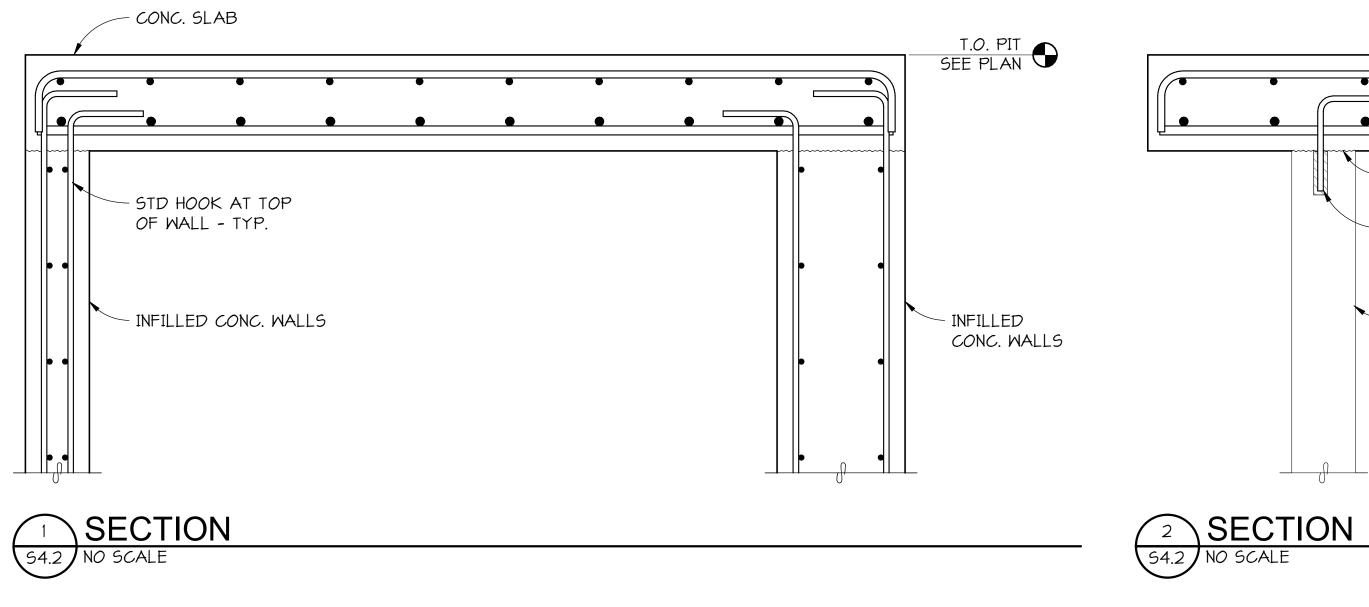


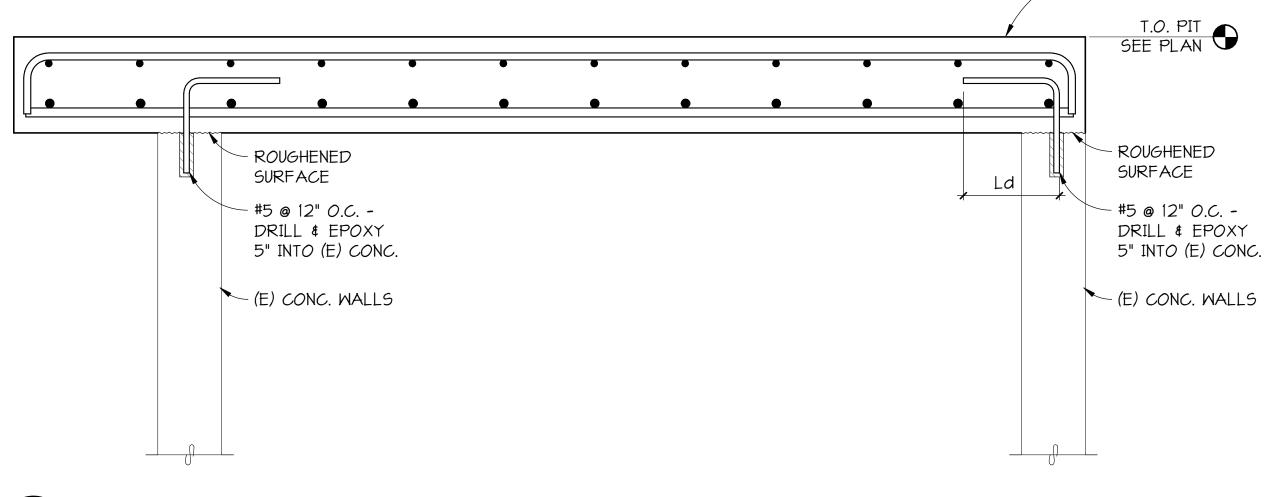


SCALE: 1" = 1'-0" PROJECT NO.: 21-102 SHEET:

S4.1







rwaaRon Wright & Associ

Ron Wright & Associates / Architects, PS 2003 Western Ave., Suite 610 Seattle, Washington 98121-3133

- CONC. SLAB

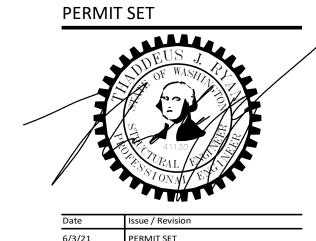
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ELEVATOR
AND STAIR
AND STAIR
REPLACEMENT
PIKE PLACE MARKET PDA
85 PIKE STREET, #500
SEATTLE, WA 98101





| Date | Issue / Revision |
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| 6/3/21 | PERMIT SET |
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SCALE: 1" = 1'-0"

PROJECT NO.: 21-102

FILE:

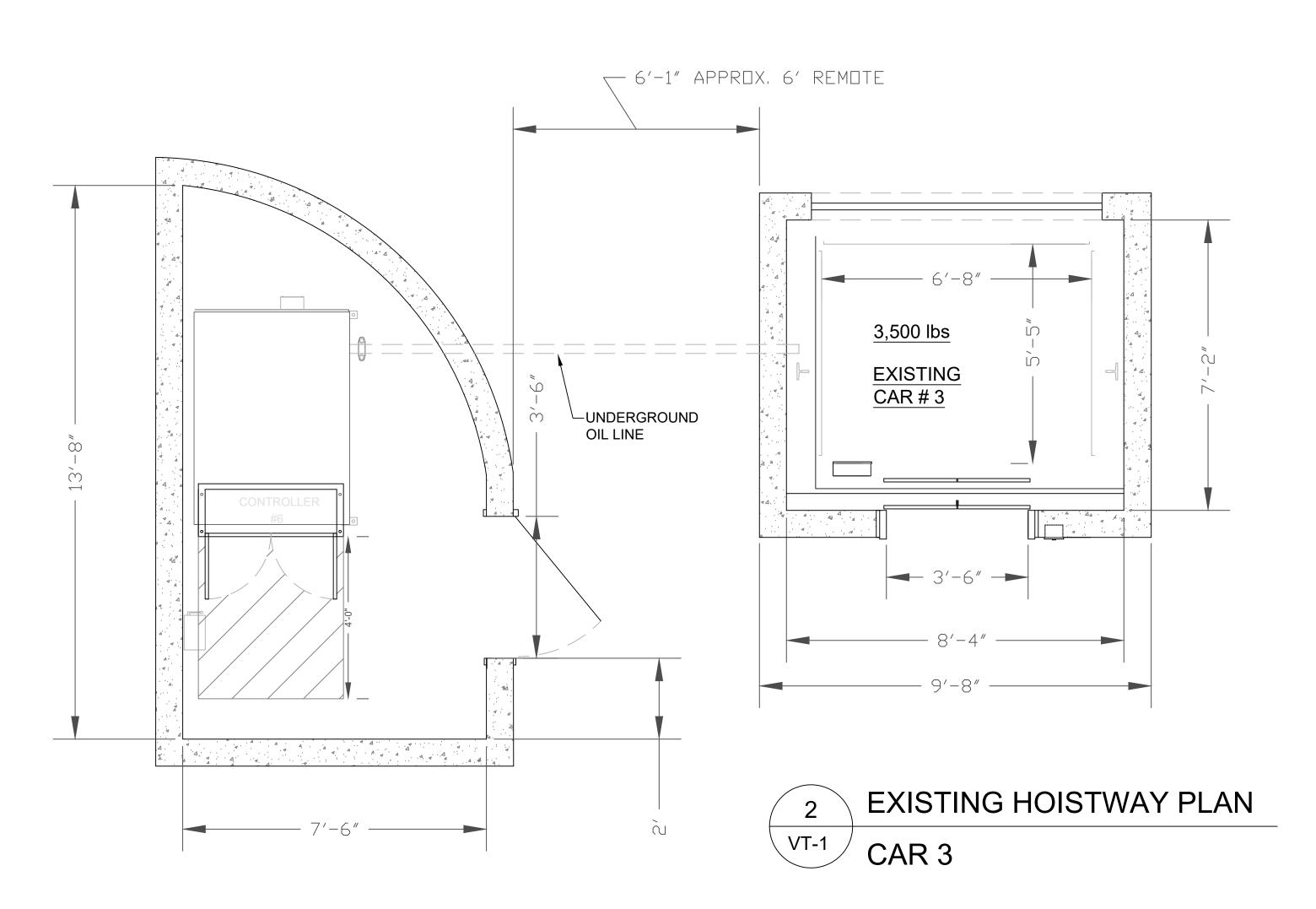
DEMOLITION NOTES:

PHASE I: DECOMMISSION BY LICENSED ELEVATOR CONTRACTOR:

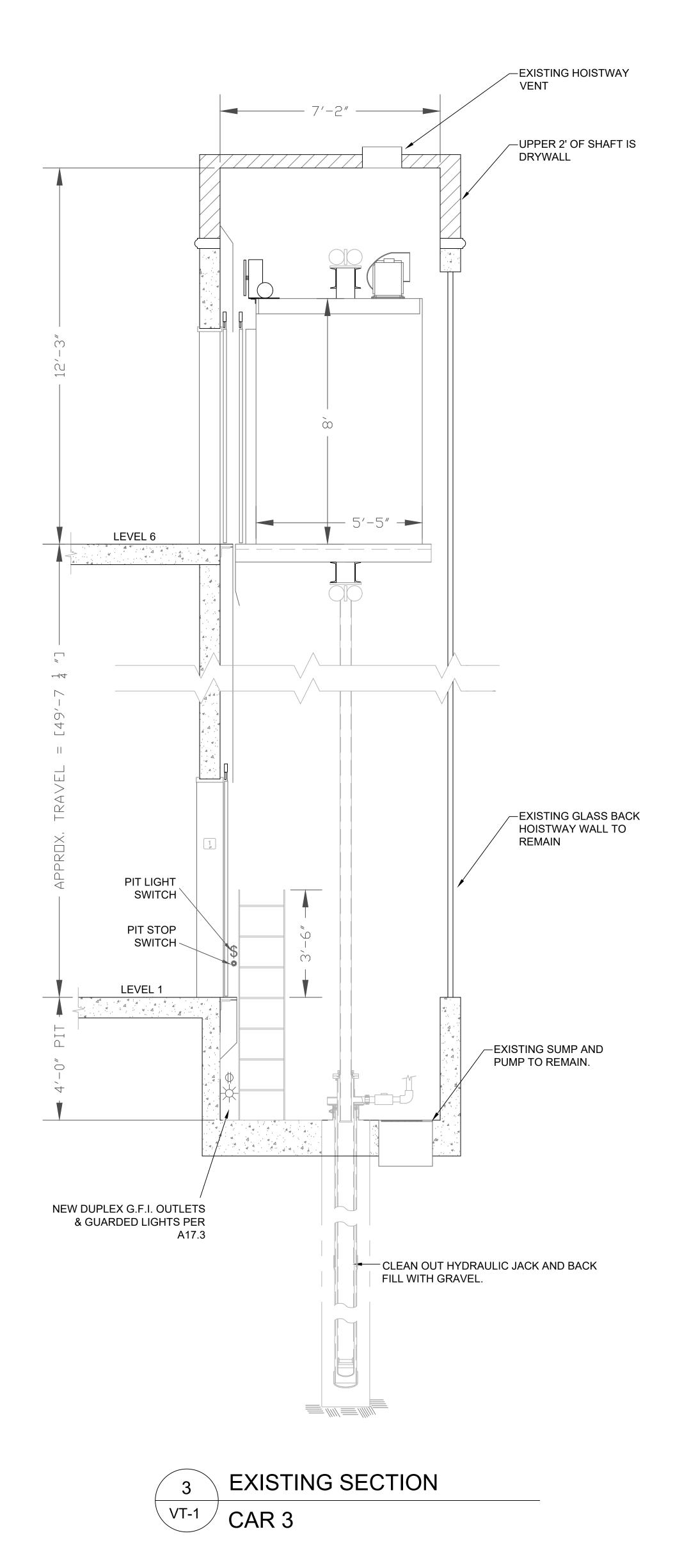
- 1. FILE PERMIT WITH CITY OF SEATTLE FOR A FORMAL DECOMMISSION.
- 2. LOCK AND SECURE ALL HOISTWAY DOORS FROM INSIDE THE SHAFT FROM TOP OF CAR. START AT TOP AND WORK TO BOTTOM.
- 3. LOWER CAR ONTO CAR BUFFERS.
- 4. MAKE A BREAK IN THE OIL PIPE LINE IN MACHINE ROOM, CAP BOTH ENDS
- 5. PHYSICALLY REMOVE WIRES FROM DISCONNECT TO CONTROLLER.
- 6. CALL FOR FINAL INSPECTION. PROVIDE FINAL PERMIT FROM CITY VERIFYING ELEVATOR HAS BEEN FORMALLY DECOMMISSIONED.

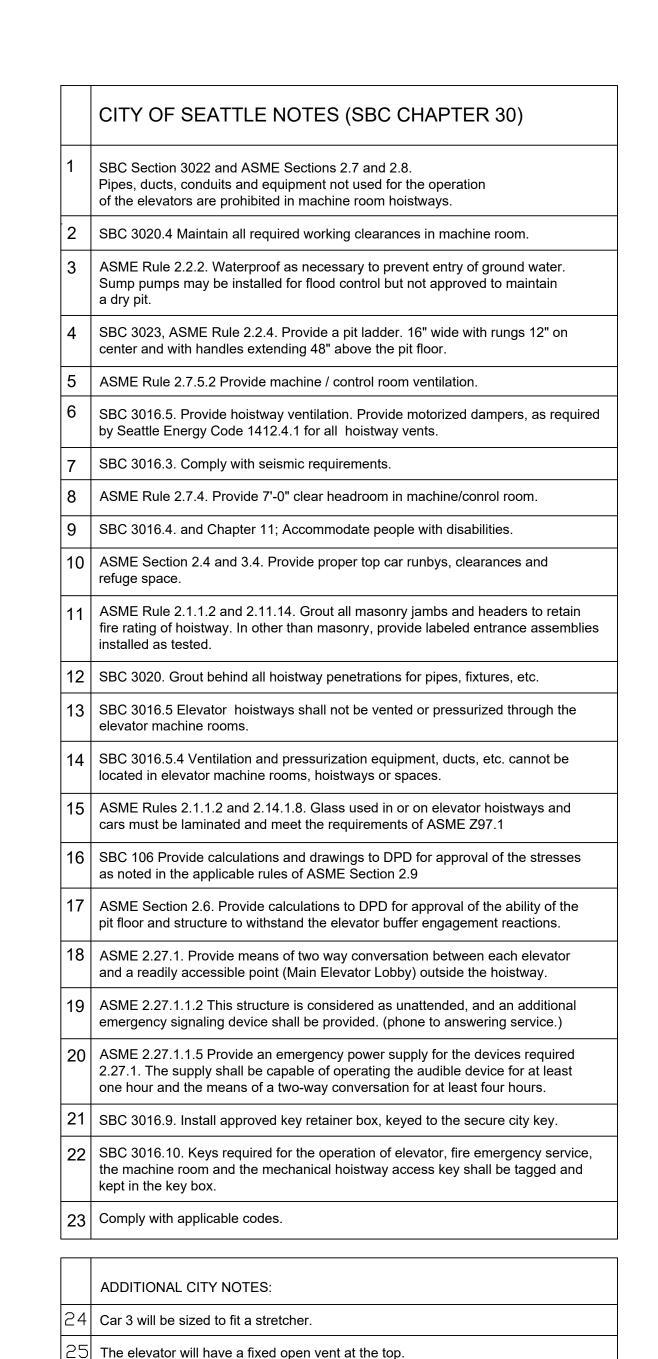
PHASE II: DEMOLITION BY LICENSED G.C./DEMO CONTRACTOR:

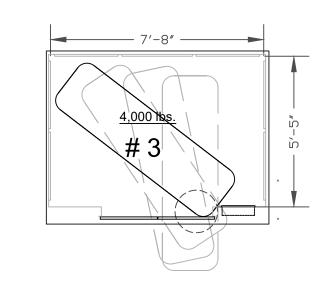
- 1. PROVIDE FULL HEIGHT LOCKABLE BARRICADES AT EACH FLOOR ON NORTH AND EAST SIDE OF ELEVATOR SHAFT. BARRICADES SHALL BE LOCKABLE FROM INSIDE AND OUTSIDE THE SHAFT. BARRICADES SHALL BE 2' IN FRONT OF EXISTING DOOR OPENINGS ON NORTH ELEVATION AND 2' WIDER THAN THE NEW WALL TO BE INSTALLED ON THE EAST ELEVATION.
- 2. ONCE BARRICADES ARE IN PLACE AT EACH FLOOR AND ALL DEMO AREAS ARE SECURED FROM THE PUBLIC CALL FOR INSPECTION WITH CITY OF SEATTLE TO VERIFY THAT AREA IS PROPERLY SECURE AND THAT DEMO BY A NON-LICENSED ELEVATOR CONTRACTOR IS PERMISSIBLE. .
- 3. REMOVE CAB, SLING, PLATFORM CAR RAILS, PIT BUFFERS LADDER FROM HOISTWAY
- 4. INSTALL FULL HEIGHT SCAFFOLDING IN HOISTWAY TO BE USED FOR EGRESS AND FOR CONSTRUCTION OF NEW ELEVATOR.
- 5. REMOVE ALL HOISTWAY DOORS AND FRAMES AT EACH FLOOR.
- 6. REMOVE ALL ELEVATOR EQUIPMENT AND DISCONNECT IN THE ELEVATOR MACHINE ROOM.











Refer to structural drawings for rail, machine, and pit floor supports.

All work will be in accordance with A17.1-2019 as adopted by the City.

NEW ELEVATOR WILL MEET IBC STRETCHER



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PIKE PLACE MARKET GARAGE ACCESS

OWNER/ DEVELOPER :
PIKE PLACE MARKET

| Date | Issue / Revision |
|----------|------------------|
| 11/6/17 | SCHEMATIC DESIGN |
| 05/25/18 | CONSTRUCTION SET |
| 11/30/21 | PLAN REVIEW SET |
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VERTICAL
TRANSPORTATION
EXISTING CAR
3 DEMO PLAN

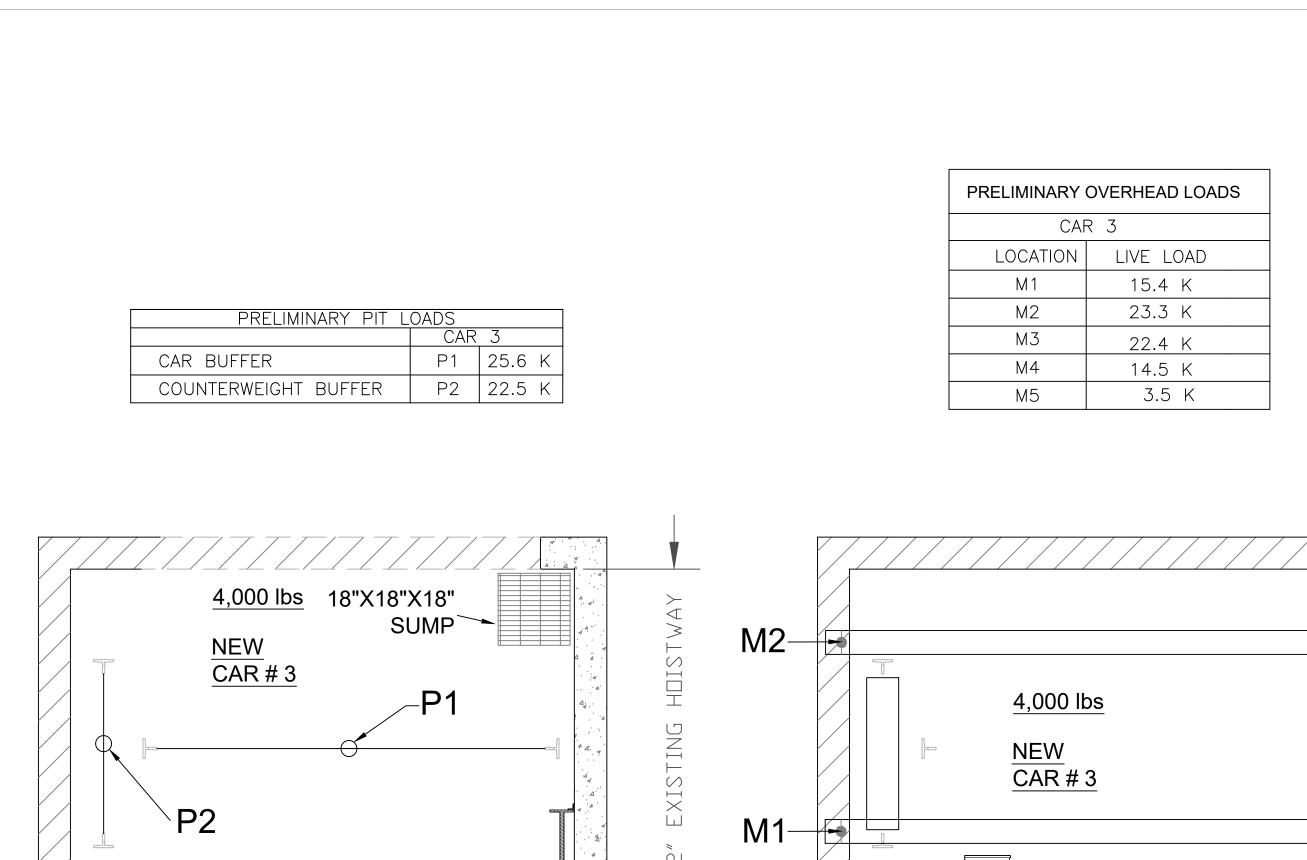
CONTENTS:

PROJECT NO.:
FILE: FILE PATH
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VT1

SHEET

VERTICAL TRANSPORTATION



10'-6" CLEAR HOISTWAY --

PIT PLAN

CAR 3

VT-2

-M3

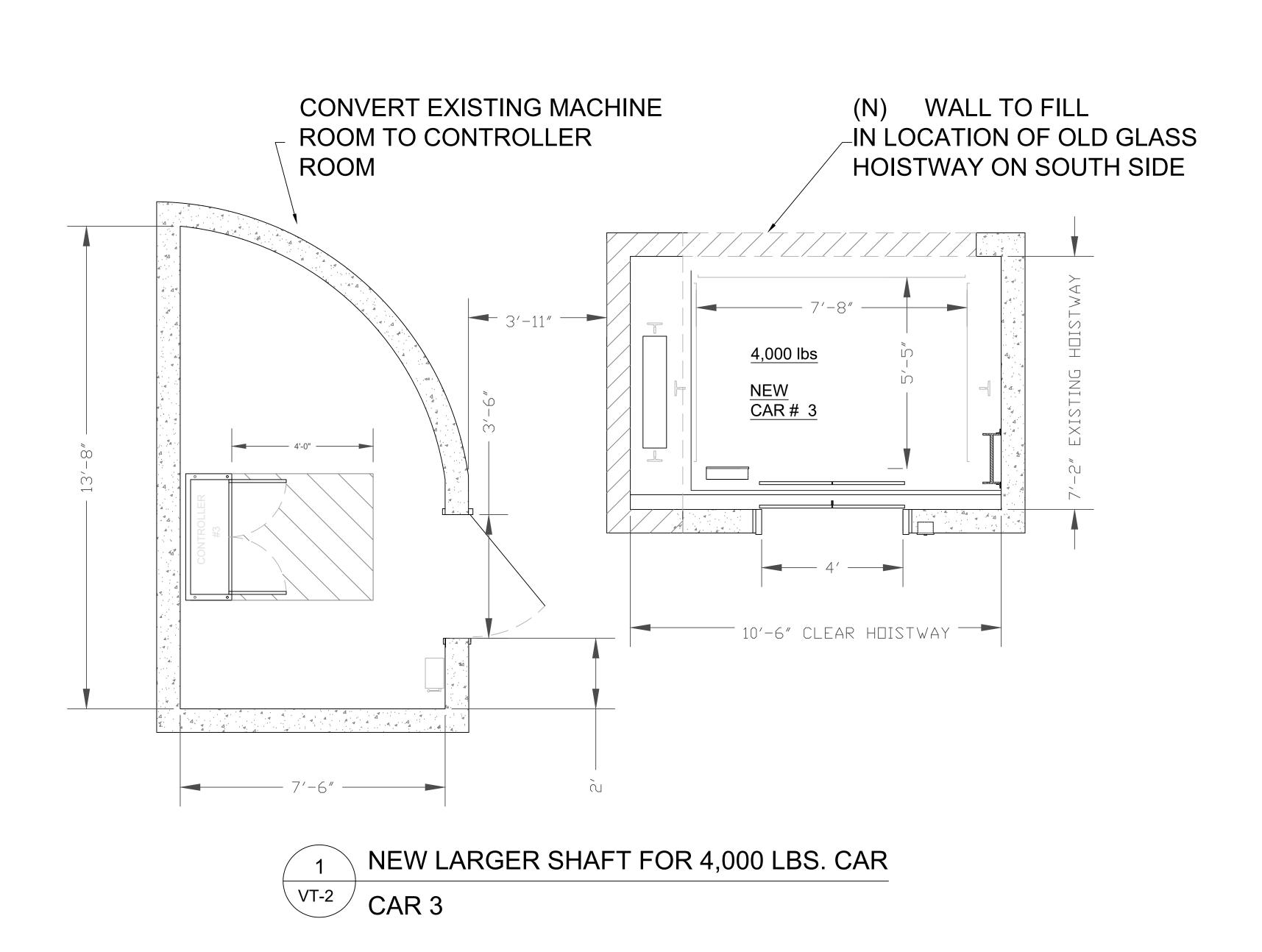
-M4

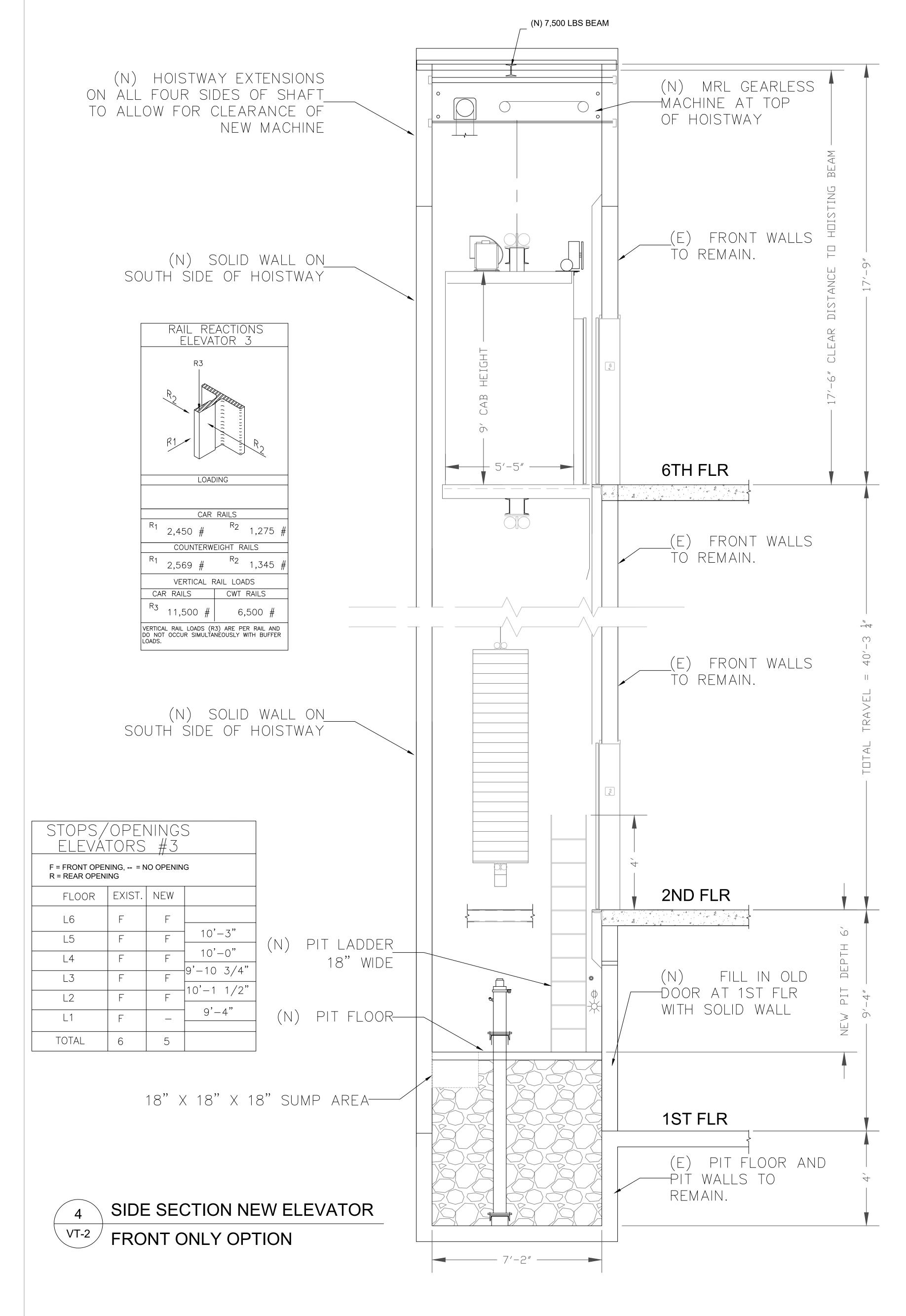
10'-6" CLEAR HOISTWAY

CAR 3

VT-2

OVERHEAD PLAN





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FAX (206) 728-4272

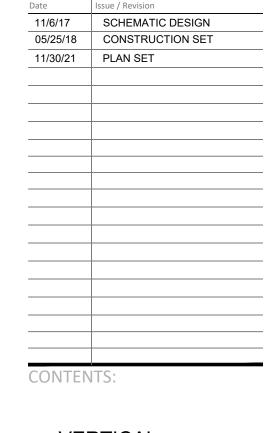
ARCHITECTURAL ELEVATOR CONSULTING, LLC

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1326 FIFTH AVE., STE. 630
SEATTLE, WA 98101
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F. 206.624.7226

PIKE PLACE MARKET GARAGE ACCESS

OWNER/ DEVELOPER :
PIKE PLACE MARKET



VERTICAL TRANSPORTATION NEW CAR 3

PROJECT NO.:

FILE: FILE PATH

SHEET: VT2

SHEET